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## Final Methodology Memorandum

| Date: | August 11, 2015 | Project \#: 18547 |
| :--- | :--- | :--- |
| To: | Devin Hearing, ODOT Region 4 \& Rick DuMilieu, Lake County |  |
| From: | Matt Kittelson, PE |  |

This memorandum documents the methodology and key assumptions to be used in preparation of the existing and future conditions analyses for the Lake County and City of Paisley Transportation System Plan (TSP) Update. The methodologies included in this memorandum are based on guidance provided in the Oregon Department of Transportation (ODOT) Transportation System Plan Guidelines (2008) and the Analysis Procedures Manual (APM), Versions 1 and 2 as they relate to rural counties in southern Oregon.

## STUDY INTERSECTIONS

Per the scope of work (SOW) intersection turning movement traffic counts will be collected at the intersections listed in Table 1. These counts are expected to be collected during May 2015. The locations for these intersection counts were agreed upon by ODOT, the County, and the consultant team during the development of the project scope. The counts will be 2-hour intersection turning movement counts and will be used to provide pedestrian volumes, bicycle volumes, truck volumes, passenger car volumes, and various calculation factors.

Table 1. Study Intersections (Location of 2-Hour Intersection Turning Movement Count)

| ID Number | East-West Name | North-South Name |
| :---: | :--- | :--- |
| 1 | OR 31 | US 395 |
| 2 | OR 140 (east leg) | US 395 |



Figure 1: Study Intersections

## PEAK HOUR DEVELOPMENT

The study intersections are over 17 miles apart. Therefore, the application of a system peak hour would not be meaningful. We propose to analyze the intersections based on their respective intersection peak hours.

## INTERSECTION OPERATIONAL STANDARDS

Per the project scope, we will present the following performance thresholds for the study intersections, regardless of jurisdictional control:

- Volume-to-capacity (v/c) ratio;
- Level-of-service (LOS);
- Delay;
- $95^{\text {th }}$ Percentile queuing (not-simulation based); and
- Turning movement counts.

This information will be provided in tables, figures, and/or technical appendices, but where possible will be provided in figures to give the general public a more clear and relatable understanding of the analysis results.

## ODOT Facilities

For reference, this section summarizes the applicable performance thresholds for study intersections that fall within ODOT's jurisdiction.

ODOT assesses intersection operations based on volume-to-capacity (V/C) ratio. Table 6 of the Oregon Highway Plan (OHP) provides volume-to-capacity targets for facilities outside the Metro area. The OHP ratios are used to evaluate existing and future no-build conditions, while Table 10-2 of the ODOT 2012 Highway Design Manual (HDM) provides V/C ratios used to assist in identifying future system deficiencies and evaluating future alternatives on state highways.

The mobility targets for the study intersections shown in Table 1 are:

- OR 31/US $395-0.70 \mathrm{v} / \mathrm{c}$ (OHP), 0.60 (HDM)
- OR 140 (east leg)/US $395-0.70 \mathrm{v} / \mathrm{c}$ (OHP), 0.60 (HDM)


## SEASONAL ADJUSTMENT FACTOR

$30^{\text {th }}$ highest hour design volumes will be based on applicable adjustment factors. Version 2 of the APM identifies three methods for identifying seasonal adjustment factors for highway traffic volumes. All three methods utilize information provided by Automatic Traffic Recorders (ATR) located in select
locations throughout the State Highway System that collect traffic data 24 -hours a day/365 days a year. There are three permanent ATR stations in Lake County (locations shown in Figure 2):

- ATR 19-004: Located on US 395, 0.26 miles south of Lakeview-Burns Highway
- ATR 19-008: Located on US 395, 0.30 miles north of Oregon-California State Line
- ATR 19-010: Located on OR 31, 2.25 miles southeast of $1^{\text {st }}$ Street;

Based on the locations of ATR stations in Lake County, a combination of the On-Site ATR method and the ATR Characteristic Table Method will be used to calculate volumes at study intersections.


Figure 2 - ATR Locations in Lake County

## On-Site ATR Method

The On-Site ATR Method requires that the ATR be located within or near the project area. If the ATR is located outside the project area, there should be no major intersections between the ATR and the project area, and the Average Annual Daily Traffic (AADT) collected by the ATR must be within 10 percent of the AADT near the project area. ODOT's Transportation Volume Tables will be used to
identify AADT for highway segments. Based on these requirements, one ATR station in Lake County can be used to calculate seasonal adjustment factors.

- ATR 19-010 can be used for nearby highway segments on OR 31.

The seasonal adjustment factors were calculated following the process outlined in the Version 2 APM, as summarized in Appendix A. The recommended seasonal adjustment factors are summarized in Table 2.

Table 2. On-Site ATR Method Seasonal Adjustment Method

| ATR Station | Weekly Traffic <br> Trend | May Seasonal <br> Adjustment Factor | Roadway Applied To |
| :---: | :---: | :---: | :---: |
| ATR 19-008 | Weekday | 0.93 | OR 31 |

## ATR Characteristic Table Method

The ATR Characteristic Table Method is proposed to calculate seasonal adjustment factors along OR 140 and county roads to the north. The Characteristic Table Method requires:

1) The ATR must be located on a facility that shares similar characteristics with the facility to be adjusted, such as seasonal traffic trends, area type, and number of lanes.
2) AADT collected by the ATR must be within 10 percent of the AADT near the project area.

ATR station 18-017 was identified for the area east of US 395 based on: the seasonal traffic trend identified for this area (Summer < 2500), AADT, and traffic trends. The seasonal adjustment factors calculated for these ATRs are shown in Table 3 and will be applied to the roadways as reported in the table.

Table 3 ATR Characteristic Table Method Seasonal Adjustments

| ATR Station | Weekly Traffic <br> Trend | May Seasonal <br> Adjustment Factor | Roadways Applied To |
| :---: | :---: | :---: | :---: |
| ATR 18-017 | Weekday | 0.91 | OR 140 E of Lakeview, County Roads |

Characteristic Tables are included in Appendix 1. Monthly seasonal adjustment factors for all ATRs in Table 2 and Table 3 are included in Appendix 2.

## ATR Application

Since existing count volumes are low in Lake County and ATR values suggest a reduction in volumes for seasonal adjustment values, no modifications will be made to existing counts.

## STUDY SEGMENTS

ODOT conducted tube counts at the segment locations identified in Table 4. These tube counts will be used to conduct two-lane highway capacity analysis using HCM 2010 methodologies. The tube counts did not contain vehicle classification information and therefore cannot be used to calculate the percentage of heavy vehicles using the roadways.

Table 4. Study Segments (48-Hour Tube Count Locations)

| Roadway Name | County Type |
| :---: | :---: |
| - OR31/Fort Rock Road | 48 hour |
| - OR31/Bear Flat Ln (cutoff between US97 and OR31 from the south) | 48 hour |
| OR140/Plush-Adel Road \& Twenty Mile Road - OR140 0.10 miles east of Plush-Adel Road <br> - OR140 0.10 miles west of Plush-Adel Road | 48 hour |
| - Plush-Adel Road \& Plush Cutoff Road <br> - North Leg | 48 hour |

## ANALYSIS MODEL PARAMETERS

The bullets below identify the proposed sources of data and methodologies to be used to analyze traffic conditions in Lake County. Analyses of all state facilities will be conducted according to the most-recent version of the APM, unless otherwise agreed upon by both ODOT's Transportation Planning and Analysis Unit (TPAU) and the consultant team.

1. Intersection/Roadway Geometry (lane numbers and arrangements, cross-section elements, signal phasing, etc.) will be verified for consistency with previous work efforts, reviewed through aerial photography, and confirmed through a site visit. Available as-built data may also be used to verify existing roadway geometry. The analysis models will be built on scaled roadway line work from GIS or aerial photography in Vistro analysis software. ODOT's twoway stop-controlled intersection calculator tool will be used to calculate expected queue lengths for two-way stop-controlled intersections.
2. Operational Data (such as posted speeds, intersection control, parking, right-turn on red, etc.) will be field verified. Data will be reviewed during a site visit and supplemented by available GIS data, aerials, photos, and the ODOT Video Log.
3. Peak Hour Factors (PHF) will be calculated for each intersection and applied to the existing conditions analyses. PHFs of 0.95 will be used for the future analysis for high-order facilities (arterials), with 0.90 applied to medium-order facilities (collectors) and 0.85 applied to local roads. If the existing PHF is greater than these default future values, the existing PHF will be applied.
4. Traffic Operations
a. The 2010 Highway Capacity Manual (HCM) methodology shall be used for intersection analyses of the design hour conditions. The existing and future no-build analysis will utilize Vistro software for all study intersections. Roundabouts (if applicable) will be analyzed using HCM 2010 analysis methods. Level-of-service, delay, and volume-tocapacity ratios will be reported at each of the study intersections regardless of roadway jurisdiction.
b. Queuing analysis methodology will be based on Vistro $95^{\text {th }}$ percentile queue lengths as appropriate; ODOT's two-way stop-controlled intersection calculator tool will be used to estimate queue lengths for two-way stop-controlled intersections. Microsimulation is not proposed as part of the long-range planning effort.

## TRAFFIC ANALYSIS SOFTWARE AND INPUT ASSUMPTIONS

Vistro software will be used for the intersection analysis. The reported results will be the level of service, intersection delay, $\mathrm{v} / \mathrm{c}$ ratios, and $95^{\text {th }}$ percentile queue lengths generated by the HCM report. None of the study intersections are signalized intersections; therefore no parameters have been provided for signal timing. Analysis assumptions are listed in Table 5.

Table 5. Operations Parameters/Assumptions

| Arterial Intersection Parameters | Existing Conditions |
| :--- | :--- |
| Peak Hour Factor | From traffic counts |
| Conflicting Bikes and Pedestrian per Hour | From traffic counts, as available |
| Ideal Saturation Flow Rate (for all movements) | 1,750 passenger cars per hour green per lane |
| Lane Width | 12 feet unless field observations suggest otherwise |
| Percent Heavy Vehicles | From traffic counts by movement, as available |
| Bus Blockages | None |
| 95th percentile vehicle queues | Vistro HCM summary output |

## CRASH ANALYSES

The most recent five years (2009 through 2013) of crash data will be reviewed at the study intersections and study segments (where tube count data was collected). Any state highways in Lake County that are identified as a Safety Priority Index System site will be included in the crash data. The data will be analyzed for a variety of factors to include type, severity, general conditions, and location to identify potential crash patterns or anomalies. Additional details will be provided on countywide crash trends and any issues that are identified through the overall review at the County, corridor/segment, and intersection level, and will include specific details on fatalities and crashes involving pedestrians and bicyclists.

Intersection crash rates will be calculated and compared to statewide crash rate performance thresholds to determine which segments or intersections have crash rates higher than similar
facilities. Given the limited number of study intersections to be studied, calculation of a critical crash rate based on the Highway Safety Manual methodology is not a reliable method for identifying a safety performance threshold. Therefore, we will use the established crash rate performance threshold based on the $90^{\text {th }}$ percentile crash rates for statewide rural intersections by traffic control type as documented in Exhibit 4-1 of the APM. Crash patterns and potential countermeasures/safety improvements will be identified and presented at intersections that exceed the statewide crash rate performance threshold.

## FORECAST YEAR VOLUME DEVELOPMENT

We developed 20-year growth factors using ODOT's historical trends method, which relies on traffic volumes from previous years to develop a growth pattern for use in projected future volumes. ODOT maintains Future Volumes Tables that summarize current and future year traffic volumes for state roadways throughout the State. To calculate the growth rate for Lake County, all Lake County locations were selected from the Future Volumes Tables. Based on guidance from ODOT's Analysis Procedures Manual (APM), data with an R-squared value (RSQ, a measure of fit) of less than 0.75 was not used. The growth rates of the remaining locations were averaged to develop an annual growth rate of $0.21 \%$. We propose to use $0.25 \%$ to project future traffic volumes at all study intersections and segments. Table 6 shows the ODOT Future Volumes Table.

## NON-AUTOMOBILE TRANSPORTATION ANALYSIS

Per the scope, the non-automobile transportation analysis will include a review of collector and arterial roadways to identify deficiencies (availability of sidewalks and bicycle lanes, and gaps in primary routes) based on available GIS data and online mapping.

Table 6. ODOT Future Volume Table (Lake County Locations with RSQ > 0.75)

| HWY | MP | Description | 2010 | 2033 | RSQ | $\begin{gathered} \text { RSQ* }> \\ 0.75 ? \end{gathered}$ | Calculated Growth Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 019 | 18.21 | Klamath-Lake County Line, 0.59 mile south of Mowich Spring Road (NF-2516) | 870 | 950 | 0.0278 | N | 0.38\% |
| 019 | 49.60 | Silver Lake Automatic Traffic Recorder, Sta. 19-010, 2.25 miles southeast of 1st Street | 710 | 720 | 0.2030 | N | 0.06\% |
| 019 | 62.95 | Picture Rock Pass Summit | 530 | 540 | 0.1753 | N | 0.08\% |
| 019 | 97.60 | 0.02 mile north of Red House Lane | 470 | 480 | 0.4341 | N | 0.09\% |
| 019 | 98.34 | 0.02 mile north of Mill Street | 750 | 760 | 0.6562 | N | 0.06\% |
| 019 | 98.38 | 0.02 mile south of Mill Street | 870 | 880 | 0.8171 | Y | 0.05\% |
| 019 | 98.89 | East city limits of Paisley, 0.11 mile south of Murphy Lane | 590 | 600 | 0.8905 | Y | 0.07\% |
| 019 | 120.37 | 0.20 mile north of Lakeview-Burns Highway (US395) | 590 | 600 | 0.3833 | N | 0.07\% |
| 019 | 120.83 | Valley Falls Automatic Traffic Recorder, Sta. 19-004, 0.26 mile south of Lakeview-Burns Highway No. 49 (US395) | 770 | 780 | 0.7938 | Y | 0.06\% |
| 019 | 138.29 | 0.05 mile north of Warner Highway (OR140) | 820 | 830 | 0.6141 | N | 0.05\% |
| 019 | 138.39 | 0.05 mile south of Warner Highway (OR140) | 1300 | 1400 | 0.1706 | N | 0.32\% |
| 019 | 140.71 | 0.02 mile north of Goldmohr Terrace Road (Sunnyslope Lane) | 1500 | 1600 | 0.8823 | Y | 0.28\% |
| 019 | 142.60 | 0.02 mile north of N. 9th Street | 1800 | 1900 | 0.8562 | Y | 0.24\% |
| 019 | 142.88 | 0.02 mile north of N. 6th Street | 1900 | 2000 | 0.9716 | Y | 0.22\% |
| 019 | 143.01 | 0.02 mile north of Klamath Falls-Lakeview Highway (OR140) | 2700 | 2800 | 0.9002 | Y | 0.16\% |
| 019 | 143.06 | 0.03 mile southeast of Klamath Falls-Lakeview Highway (OR140) | 5200 | 5300 | 0.7819 | Y | 0.08\% |
| 019 | 143.32 | 0.02 mile south of Center Street | 4200 | 4300 | 0.9169 | Y | 0.10\% |
| 019 | 143.65 | 0.02 mile north of 4th Street South | 3700 | 3800 | 0.9070 | Y | 0.12\% |
| 019 | 143.86 | 0.02 mile north of 7th Street South | 3300 | 3400 | 0.8535 | Y | 0.13\% |
| 019 | 144.08 | 0.02 mile south of "F" Street South | 2200 | 2300 | 0.8627 | Y | 0.19\% |
| 019 | 144.22 | 0.02 mile south of 10 th Street South | 2000 | 2200 | 0.0591 | N | 0.42\% |
| 019 | 144.47 | 0.02 mile south of 12th Street South | 1400 | 1600 | 0.0174 | N | 0.58\% |
| 019 | 148.63 | 0.02 mile north of Crane Creek Road | 1000 | 1100 | 0.2190 | N | 0.42\% |
| 019 | 157.43 | New Pine Creek Automatic Traffic Recorder, Sta. 19-008, 0.30 mile north of Oregon-California State Line | 940 | 950 | 0.6874 | N | 0.05\% |
| 020 | 70.73 | 4.00 miles southeast of Quartz Mountain Pass Summit | 760 | 790 | 0.0413 | N | 0.17\% |
| 020 | 88.96 | 0.10 mile west of Tunnel Hill Road | 1000 | 1100 | 0.1554 | N | 0.42\% |
| 020 | 89.16 | 0.10 mile east of Tunnel Hill Road | 1100 | 1200 | 0.3184 | N | 0.38\% |
| 020 | 92.43 | 0.70 mile east of Westside Road at Maddock Corner | 1400 | 1500 | 0.3251 | N | 0.30\% |
| 020 | 93.88 | 0.02 mile west of road to Airport | 1600 | 1700 | 0.8518 | Y | 0.26\% |
| 020 | 95.39 | 0.02 mile east of Roberta Avenue | 2100 | 2200 | 0.7302 | N | 0.20\% |
| 020 | 95.72 | 0.02 mile east of N. "R" Street | 2600 | 2700 | 0.8530 | Y | 0.16\% |
| 020 | 96.03 | 0.02 mile west of "L" Street | 3100 | 3200 | 0.8341 | Y | 0.14\% |
| 020 | 96.35 | 0.02 mile west of Fremont Highway (US395) | 3000 | 3100 | 0.8492 | Y | 0.14\% |
| 049 | 0.20 | 0.19 mile south of Central Oregon Highway (US20) | 380 | 500 | 0.7461 | Y | 1.20\% |
| 049 | 89.52 | 0.50 mile north of Fremont Highway (OR31) | 270 | 280 | 0.3691 | N | 0.16\% |
| 431 | 0.10 | 0.10 mile east of Fremont Highway (US395) | 550 | 580 | 0.3165 | N | 0.23\% |
| 431 | 8.07 | 0.02 mile west of Summit Prairie Road | 530 | 570 | 0.3385 | N | 0.32\% |
| 431 | 15.81 | 0.02 mile east of Plush Cutoff Road | 420 | 440 | 0.1447 | N | 0.20\% |
| 431 | 28.08 | 0.10 mile west of Twenty Mile Road | 310 | 320 | 0.1032 | N | 0.14\% |
| 431 | 28.28 | 0.10 mile east of Twenty Mile Road | 280 | 290 | 0.0005 | N | 0.15\% |
| 431 | 65.28 | Oregon-Nevada State Line | 180 | 190 | 0.1552 | N | 0.24\% |
| Average Growth Rate: |  |  |  |  |  |  | 0.21\% |

*RSQ = R-squared value, which describes the fit of the data to a line.

## Appendix 1 On-site ATR Characteristics

| ATR CHARACTERISTIC TABLE (Printed: 09/18/14 ) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SEASONAL TRAFFIC TREND | AREA TYPE | $\begin{gathered} \text { \# OF } \\ \text { LANES } \end{gathered}$ | WEEKLY TRAFFIC TREND | AADT | OHP CLASSIFICATION | ATR | COUNTY | highway route, NAME, \& LOCATION | MP | STATE HWY NUMBER |
| SUMMER < 2500 | RURAL | 2 | WEEKDAY | 800 | DISTRICT HWY | 01-001 | BAKER | US30, LA GRANDE-BAKER HIGHWAY, 4.84 MILES SOUTH OF UNION BAKER COUNTY LINE | 37.70 | 66 |


| ATR CHARACTERISTIC TABLE (Printed: 09/18/14 ) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SEASONAL TRAFFIC TREND | AREA TYPE | \# OF <br> LANES | WEEKLY TRAFFIC TREND | AADT | OHP CLASSIFICATION | ATR | COUNTY | HIGHWAY ROUTE, NAME, \& LOCATION | MP | STATE HWY NUMBER |
| RECREATIONAL SUMMER | RURAL | 2 | WEEKEND | 890 | STATEWIDE HWY | 18-017 | KLAMATH | OR140, KLAMATH FALLSLAKEVIEW HIGHWAY, 4.14 MILES EAST OF YELLOW JACKET SPRINGS ROAD AT BEATTY | 44.92 | 20 |

## Appendix 2 ATR Summary Information

| ODOT ATR 19-004: US395; MP 120.83; Freemont Highway No. 19, 0.26 miles south of Lakeview-Burns Highway No. 49 (US395) |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Year | 15-Jan | 15-Feb | 15-Mar | 15-Apr | 15-May | 15-Jun | 15-Jul | 15-Aug | 15-Sep | 15-Oct | 15-Nov | 15-Dec |
| 2013 | 64 | 75 | 86 | 95 | 109 | 127 | 129 | 141 | 130 | 104 | 90 | 74 |
| 2012 | 64 | 74 | 78 | 98 | 114 | 126 | 132 | 143 | 134 | 104 | 94 | 68 |
| 2011 | 74 | 73 | 88 | 100 | 110 | 127 | 133 | 133 | 134 | 101 | 88 | 74 |
| 2010 | 68 | 81 | 85 | 89 | 104 | 121 | 141 | 138 | 141 | 117 | 87 | 70 |
| 2009 | 67 | 71 | 82 | 99 | 113 | 125 | 135 | 133 | 133 | 105 | 89 | 76 |
| Average | 66.3 | 74.0 | 84.3 | 97.3 | 110.7 | 126.0 | 133.3 | 137.3 | 133.7 | 104.3 | 89.0 | 72.7 |
| Count Adj. | 1.51 | 1.35 | 1.19 | 1.03 | 0.90 | 0.79 | 0.75 | 0.73 | 0.75 | 0.96 | 1.12 | 1.38 |

Represent min/max values removed from average

| ODOT ATR 19-008: US395; MP 157.43; Freemont Highway No. 19, 0.30 miles north of Oregon-California State Line |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Year | 15-Jan | 15-Feb | 15-Mar | 15-Apr | 15-May | 15-Jun | 15-Jul | 15-Aug | 15-Sep | 15-Oct | 15-Nov | 15-Dec |
| 2013 | 70 | 80 | 87 | 100 | 107 | 117 | 123 | 130 | 125 | 107 | 92 | 78 |
| 2012 | 76 | 78 | 82 | 94 | 109 | 119 | 125 | 132 | 128 | 104 | 94 | 75 |
| 2011 | 109 | 100 | 88 | 88 | 108 | 123 | 126 | 121 | 113 | 93 | 82 | 73 |
| 2010 | 64 | 70 | 82 | 85 | 96 | 115 | 123 | 128 | 139 | 124 | 103 | 96 |
| 2009 | 71 | 73 | 83 | 98 | 109 | 120 | 126 | 123 | 126 | 102 | 95 | 78 |
| Average | 72.3 | 77.0 | 84.0 | 93.3 | 108.0 | 118.7 | 124.7 | 127.0 | 126.3 | 104.3 | 93.7 | 77.0 |
| Count Adj. | 1.38 | 1.30 | 1.19 | 1.07 | 0.93 | 0.84 | 0.80 | 0.79 | 0.79 | 0.96 | 1.07 | 1.30 |

Represent min/max values removed from average

| ODOT ATR 01-001: US 30; MP 37.70; La Grande-Baker Highway No. 66, 4.84 miles south of Union-Baker County Line |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Year | 15-Jan | 15-Feb | 15-Mar | 15-Apr | 15-May | 15-Jun | 15-Jul | 15-Aug | 15-Sep | 15-Oct | 15-Nov | 15-Dec |
| 2013 | 79 | 94 | 98 | 109 | 111 | 116 | 142 | 128 | 117 | 111 | 101 | 90 |
| 2012 | 89 | 102 | 100 | 113 | 114 | 113 | 118 | 116 | 116 | 112 | 100 | 89 |
| 2011 | 77 | 86 | 89 | 110 | 120 | 121 | 125 | 123 | 125 | 111 | 103 | 92 |
| 2010 | 90 | 95 | 102 | 107 | 113 | 121 | 121 | 119 | 117 | 108 | 93 | 86 |
| 2009 | 80 | 89 | 94 | 107 | 118 | 131 | 120 | 120 | 114 | 108 | 100 | 94 |
| Average | 82.7 | 92.7 | 97.3 | 108.7 | 115.0 | 119.3 | 122.0 | 120.7 | 116.7 | 110.0 | 100.3 | 90.3 |
| Count Adj. | 1.21 | 1.08 | 1.03 | 0.92 | 0.87 | 0.84 | 0.82 | 0.83 | 0.86 | 0.91 | 1.00 | 1.11 |

Represent min/max values removed from average

| ODOT ATR 18-017: OR 140; MP 44.98; Klamath Falls-Lakeview Highway No. 20, 4.20 miles east of Yellow Jacket Spring Road at Beatty |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Year | 15-Jan | 15-Feb | 15-Mar | 15-Apr | 15-May | 15-Jun | 15-Jul | 15-Aug | 15-Sep | 15-Oct | 15-Nov | 15-Dec |
| 2013 | 71 | 75 | 81 | 91 | 109 | 124 | 126 | 128 | 123 | 104 | 89 | 74 |
| 2012 | 74 | 74 | 73 | 86 | 106 | 120 | 136 | 137 | 131 | 114 | 87 | 73 |
| 2011 | 88 | 94 | 92 | 86 | 116 | 123 | 122 | 126 | 116 | 99 | 79 | 76 |
| 2010 | 60 | 65 | 70 | 75 | 95 | 129 | 145 | 140 | 139 | 120 | 94 | 77 |
| 2009 | 67 | 69 | 74 | 87 | 113 | 125 | 125 | 130 | 125 | 114 | 88 | 79 |
| Average | 70.7 | 72.7 | 76.0 | 86.3 | 109.3 | 124.0 | 129.0 | 131.7 | 126.3 | 110.7 | 88.0 | 75.7 |
| Count Adj. | 1.42 | 1.38 | 1.32 | 1.16 | 0.91 | 0.81 | 0.78 | 0.76 | 0.79 | 0.90 | 1.14 | 1.32 |

Represent min/max values removed from average

