# TECHNICAL MEMORANDUM \#4 

Date: May 9, 2023
Project \#: 23021.050
To: Project Management Team
From: Kittelson \& Associates, Inc.
Project: Curry County Transportation System Plan Update
Subject: Final Tech Memo \#4: Current Transportation System Operations (Task 5.1)

## INTRODUCTION

The current transportation system operations summarized in this memorandum reflect how Curry County's transportation system performs as it exists today in terms of intersection traffic operations, crash history, conflicts for freight, and multimodal conditions. The assessment summarized herein is based on information gathered and assembled from Geographic Information System (GIS) data and aerial imagery, measured traffic data, and historical crash rates provided or produced by the County and the Oregon Department of Transportation (ODOT). Much of the inventory and analysis results are presented in figures and tables and are supplemented with text.

This assessment identifies gaps and deficiencies in the existing transportation system that will be used to help inform the policies, projects, programs and/or studies recommended in the Curry County Transportation System Plan (TSP) Update. The TSP addresses transportation needs for people walking, rolling, taking transit, biking, and driving within Curry County, primarily in the rural areas, but also along County facilities within Urban Growth Boundaries (UGBs) of incorporated cities. The geographic extents of the TSP study area are shown in Figure 1.

The information provided in this memorandum addresses the requirements identified in Oregon Administrative Rule 660-012-020 (Elements of a Transportation System Plan) for providing a general assessment of existing transportation facilities and services.


gure 1
Bureau of Land Management
dy Area

- County Boundary

Study Area - State Border


## EXECUTIVE SUMMARY

Key findings from the current transportation system operations assessment presented within this memorandum are summarized below.

## Transportation System Operations Findings

- All study intersections currently meet their applicable mobility targets during the evening peak hour (the US 101 / Winchuck River Road-Ocean View Drive experiences the highest side-street delay).
- All available vehicle storage is adequate to serve the current traffic volume queves.
- Non-motorized pedestrian and bicycle movements are generally low at the study intersections.


## Crash Analysis Findings

- 928 crashes were reported in Curry County between 2017 and 2021.
- $39 \%$ of all reported crashes were with a fixed or other object, $19 \%$ were turning movement, and $15 \%$ were rear-end.
- 22 of all reported crashes included pedestrians and 8 included bicyclists.
- $59 \%$ of all reported crashes resulted in some level of injury, including 14 fatal crashes and 45 serious injury crashes ( 4 of the fatal crashes included pedestrians and one included a bicyclist).
- No crashes were reported at the US 101 / Cape Blanco Highway, US 101 / Pistol River Road, or US 101 / Cape Ferrelo Road intersections during the study period.
- No study intersection has an observed crash rate that exceeds the applicable 90 ${ }^{\text {th }}$ percentile crash rate or critical crash rate (the US 101 / Floras Creek Road intersection is approaching its critical crash rate threshold).
- No study intersection exhibits an excess proportion of any one crash type.
- 44 of the 125 study segments have observed crash rates that exceed the rural highway crash rates (many exhibited less than one crash per year).
- 6 segments, all along US 101, had more than ten total crashes, 4 of which are in Brookings (most the crashes were rear-end or turning movement and included 3 of the fatal crashes).
- There are no Safety Priority Index System (SPIS) sites in the top $10 \%$ within Curry County from the most recent SPIS list.


## Freight Analysis Findings

- US 101, OR 250, and OR 255 have freight length and width restrictions and are not authorized to move triples combinations. US 101 includes 2 weight restricted bridges. OR 250 and OR 255 are not authorized to continuously move 14 feet wide mobile homes / modular building units.
- Alternative freight routes in the county are limited and may not be appropriate for moving freight in their current condition.
- One low-priority freight pinch point on US 101 is located near Humbug Mountain State Park that would be costly to remove. No other freight pinch points or delays are identified on the State highway system in the county.
- 29 bridges are identified as either being structurally deficient, weight restricted, or having sufficiency ratings below 50 ( 10 of these bridges are County owned). No seismic bridges are identified on the State highway system in the county.
- Lower Harbor Road in Brookings and Dock Road to Harbor Drive in Port Orford are intermodal connectors with identified needs for moving freight.


## Multimodal Analysis Findings

- Most arterials and collectors, or sections of these roadways, score with a BLTS 2 or 3. Roadway BLTS scores generally lower further from the urbanized areas of the county, where traffic volumes are lower, and higher in the urbanized areas, depending on traffic volume levels, posted speeds, and availability of dedicated biking facilities.
- Most arterials and collectors result in "Poor" Pedestrian Qualitative Multimodal Assessment (QMA) ratings - except within and near the incorporated cities and unincorporated communities. US 101 is primarily rated as "Fair" near the incorporated cities and unincorporated communities and demonstrates some "Good" ratings in Port Orford and Brookings. Some sections of US 101 and the remaining State highways are rated as "Poor."
- Transit services and facilities in the county results in a Transit QMA rating of "Fair" primarily due to its frequency.
- Safety risks to bicyclists are relatively high along the US 101 corridor throughout the county but are highest within the Brookings UGB.
- The greatest safety risks to pedestrians are the highest on US 101 near Airport Road, within the city limits of Port Orford and Gold Beach (northern) and the Brookings UGB, and near the Cape Sebastian area north of Pistol River.


## CURRENT TRANSPORTATION SYSTEM OPERATIONS ANALYSIS

The current transportation system operations analysis identifies how the eight study intersections from Figure 1 operate during the weekday evening peak period, including non-motorized transportation movements. This section summarizes the resultant intersection operations and vehicle queuing. These results create a foundation for assessing possible solutions to any capacity deficiencies identified at the study intersections.

## Traffic Counts

The study intersections for the Curry County TSP Update were determined by Curry County and ODOT prior to development of the scope of work. The eight study intersections identified in Figure 1 are all located along US 101 at major County roadways, and none are within UGBs of the incorporated cities. Traffic counts were conducted at the study intersections in September 2022 on a typical weekday over a 2-hour period (4:00 to 6:00 p.m.) while school was in session. All the counts include the total number of pedestrians, bicyclists, and motor vehicles that entered the intersections in 15-minute intervals throughout the study periods. Attachment A contains the traffic count worksheets.

## Analysis Methodology and Mobility Targets

All traffic operations analyses described in the following sections are in conformance with State standard methodologies and guidelines. The study intersections were analyzed during the "30th highest hours," which generally corresponds to the July PM peak hour. Applicable volume-tocapacity (V/C) ratio targets summarized in Table 1 were compared with the operational results described in the following section. More details on the analysis methodology and assumptions, including development of intersection peak hours and seasonal adjustment factors, can be found in the Methodology \& Assumptions Memorandum.

Table 1. ODOT V/C Ratio Targets

| Map ID | Intersection | OHP Mobility Targe ${ }^{1}$ |
| :---: | :---: | :---: |
| 1 | US 101 / Floras Creek Rd | 0.75 N-S / 0.80 E |
| 2 | US 101 / Sixes River Rd | 0.70 N-S / 0.75 E |
| 3 | US 101 / Cape Blanco Hwy | 0.70 N-S / 0.75 W |
| 4 | US 101 / Ophir Rd | 0.70 N-S / 0.75 E |
| 5 | US 101 / Edson Creek Rd-Nesika Rd | 0.70 N-S / 0.75 E-W |
| 6 | US 101 / Pistol River Rd | 0.70 N-S / 0.75 E |
| 7 | US 101 / Cape Ferrelo Rd | 0.70 N-S / 0.75 E |
| 8 | US 101 / Winchuck River Rd | 0.75 N-S / 0.80 E-W |

IState Highway V/C Ratio / Side-Street V/C Ratio

## Current Traffic Operations Analysis

The current traffic operations analysis helps to identify if study intersections exceed their V/C ratio targets today. The analysis used Vistro software and its Highway Capacity Manual (HCM Reference 1) $7^{\text {th }}$ Edition reports to summarize V/C ratios, Levels of Service (LOS), delay, and $95^{\text {th }}$ percentile queues at the study intersections. Figure 2 illustrates the current traffic control devices and lane configurations at the study intersections.

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Figure 3 shows the existing traffic volumes for the study intersections and their resultant traffic operations. Figure 3 identifies the V/C ratios, LOS, and delay at the study intersections and indicates if the applicable V/C ratio targets from Table 1 are met. The V/C ratios shown are reported for the critical movement at the intersections provided that they are all stop-controlled. Last, Table 2 summarizes the $95^{\text {th }}$ percentile queues at the study intersections as compared to their available vehicle storage.

Table 2. $95^{\text {th }}$ Percentile Queuing

| Map ID | Intersection | Movement ${ }^{1}$ | Storage Length (Feet) ${ }^{2}$ | 95h Percentile Queve (Feet) ${ }^{3}$ | Adequate? |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | US 101 / Floras Creek Rd | SBLT | 490 | <25 | Yes |
|  |  | WBLR | 125 | 25 | Yes |
| 2 | US 101 / Sixes River Rd | NBL | 95 | $<25$ | Yes |
|  |  | SBL | 100 | $<25$ | Yes |
|  |  | EBLTR | 60 | $<25$ | Yes |
|  |  | WBLTR | 600 | 25 | Yes |
| 3 | US 101 / Cape Blanco Hwy | NBL | 180 | 25 | Yes |
|  |  | EBLR | 60 | 25 | Yes |
| 4 | US 101 / Ophir Rd | SBL | 750 | $<25$ | Yes |
|  |  | WBLR | 420 | $<25$ | Yes |
| 5 | US 101 / Edson Creek Rd-Nesika Rd | NBLTR | 980 | $<25$ | Yes |
|  |  | SBLTR | 3,400 | $<25$ | Yes |
|  |  | EBLTR | 980 | 25 | Yes |
|  |  | WBLTR | 260 | 25 | Yes |
| 6 | US 101 / Pistol River Rd | SBL | 155 | $<25$ | Yes |
|  |  | WBLR | 940 | 25 | Yes |
| 7 | US 101 / Cape Ferrelo Rd | SBLT | 760 | $<25$ | Yes |
|  |  | WBLR | 570 | 25 | Yes |
| 8 | US 101 / Winchuck River Rd | NBL | 185 | 25 | Yes |
|  |  | SBL | 205 | 25 | Yes |
|  |  | EBLTR | 70 | 25 | Yes |
|  |  | WBLTR | 300 | 25 | Yes |

${ }^{1} \mathrm{NB}=$ northbound; $\mathrm{SB}=$ southbound; $\mathrm{EB}=$ eastbound; $\mathrm{WB}=$ westbound; $\mathrm{L}=$ left; $\mathrm{T}=$ through; $\mathrm{R}=$ right
${ }^{2}$ Storage lengths reflect striped storage for each turn-lane pocket at the intersections or available storage to the upstream driveway or intersection.
${ }^{3}$ Vehicle queues were rounded to the nearest 25 feet.
As shown, all study intersections currently meet their $V / C$ ratio targets, and all available vehicle storage is adequate to serve the current traffic volume queues. Attachment B contains the existing traffic operations worksheets.

[^1]US $101 /$


US 101 /


US 101 /


US 101 /

US 101 /



US 101 /


US $101 /$

## Non-Motorized Transportation Analysis

Non-motorized transportation movements were evaluated at the study intersections during the PM peak hour and showed that non-motorized pedestrian and bicycle movements are generally low at the intersections today. The following intersections experienced some nonmotorized pedestrian and bicycle movements:

- US 101 / Sixes River Road
- West Leg: 1 pedestrian
- US 101 / Cape Blanco Highway (OR 250)
- West Leg: 1 pedestrian
- US 101 / Pistol River Road
- Northbound Through: 1 bicyclist
- Southbound Through: 4 bicyclists
- US 101 / Cape Ferrelo Road
- Northbound Through: 1 bicyclist
- US 101 / Winchuck River Road-Ocean View Drive
- North Leg: 1 pedestrian
- East Leg: 1 pedestrian

As indicated in Technical Memorandum \#3 (Update System Inventory), there are several gaps and deficiencies in the existing pedestrian and bicycle networks that limit pedestrian and bicycle movements along roadways and at intersections, including several of the intersections shown above. The current multimodal analysis described in later sections of this memorandum provides additional information related to pedestrian and bicycle facilities throughout the county.

## Transportation System Operations Findings

- All study intersections currently meet their applicable mobility targets during the evening peak hour (the US 101 / Winchuck River Road-Ocean View Drive experiences the highest side-street delay).
- All available vehicle storage is adequate to serve the current traffic volume queues.
- Non-motorized pedestrian and bicycle movements are generally low at the study intersections.


## CRASH ANALYSIS

Curry County's crash history was reviewed to identify any potential safety focus locations to be considered as part of future alternatives analyses. This review evaluated potential crash patterns throughout the county (e.g., collision type, crash severity), analyzed crash rates and crashes in excess at the study intersections, crash rates for arterials and collectors, and identified any Safety Priority Index System (SPIS) sites. The analysis is based on the most recent available five
years of reported crash data (January 1, 2017 through December 31, 2021) obtained from ODOT's Crash Analysis and Reporting Unit.

## Countywide Crash Characteristics

The following section provides an overview of crash characteristics throughout Curry County including the location, type, and severity of all crashes that occurred along City, County, and ODOT facilities within the county.

## Severity and Location

A total of 928 crashes were reported within Curry County between 2017 and 2021. Table 3 and Figure 4 present the severities of these crashes by percentage of total crashes and according to where they occurred, respectively. Fourteen fatal crashes and 45 serious injury crashes were reported during the study period. Approximately 59 percent of all reported crashes resulted in some level of injury.

Table 3. Curry County Reported Crashes by Severity (2017-2021)

|  | Fatality | Serious Injury <br> (A) | Moderate <br> Injury (B) | Minor Injury <br> (C) | Property <br> Damage <br> Only (PDO) | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Number of <br> Reported Crashes | 14 | 45 | 231 | 254 | 384 | 928 |
| Percentage of <br> Total Crashes | $2 \%$ | $5 \%$ | $25 \%$ | $27 \%$ | $41 \%$ | $100 \%$ |

Of the fourteen reported fatal crashes, four were with pedestrians, one was with a bicyclist, five were fixed-object, two were overturned vehicles, one was a turning-movement collision, and one was head-on. Five occurred in the daylight and the rest occurred in darkness or dusk. The following list provides a summary of each fatal crash:

- US 101 at MP 290.20 (north of Willow Creek Road): An overturned vehicle crash occurred in December 2018 on a cloudy, wet day in the darkness. There was one vehicle involved with two passengers, one of whom sustained a serious injury and one of whom was killed. No speeding, drugs, or alcohol were reported as involved.
- US 101 at MP 294.96 (north of Childers Road): A fixed-object crash (with a tree) occurred in October 2020 on a clear, dry day in the darkness. The cause was cited as speed too fast for the limit and involved a fire or explosion. The crash was flagged as alcohol related. The driver of the vehicle was killed.
- US 101 at MP 322.99 (north of N Chentrelle Lane): A fixed-object crash (with a tree) occurred in February 2020 in the daylight. The cause was cited as improper driving and was flagged as alcohol related. The driver of the vehicle was killed.
- US 101 at MP 339.26 (north of Carpenterville Highway): A turning movement crash occurred in June 2021 in foggy, dry conditions in the daylight. The two vehicles involved were going in the same direction - one was turning, and one was going straight. The cause was cited as improper overtaking and driving in excess speed. There was one fatality and five uninjured people. No drugs or alcohol were reported as involved.

- Moderate Injury $\square$ County Boundary
- PDO $\qquad$

- Minor Injury US Forest Service

- Minor Injury US Forest Service
- PDO - State Border
- US 101 at MP 342.40 (south of Mack Arch Road): A head-on crash occurred in May 2021 in clear, dry conditions in the daylight. The cause was cited as the driver driving on left of center on a two-way road, straddling the centerline, and colliding head-on with a vehicle from the opposing direction. The driver was killed, and in the other vehicle, one person sustained a possible injury (C), and one person was uninjured. No speeding, drugs, or alcohol were reported as involved.
- US 101 at MP 344.81 (north of Wilderness Road): A fixed-object crash (with a tree) occurred in February 2019 on a cloudy, wet day in the darkness. The driver was drowsy, and the crash was flagged as drugs related. The driver was killed.
- US 101 at MP 349.37 (south of Whaleshead Road): An angle crash occurred between a vehicle and a cyclist in February 2018 on a clear, dry day in the daylight. The cyclist did not yield the right-of-way and was struck and killed by the driver. This crash was flagged as drugs and marijuana related.
- US 101 at MP 357.49 (south of Crystal Creek Road): A pedestrian crash occurred in October 2020 in the darkness. The cause was cited as the pedestrian disregarding the traffic signal and was flagged as drugs involved. The pedestrian was struck by the driver and killed, and the driver was uninjured.
- US 101 at MP 358.27 (north of Sunshine Cove Lane): A pedestrian crash occurred in January 2021 on a cloudy, wet day in the darkness. The cause was cited as the pedestrian not having right-of-way, inattention, and not wearing reflective clothing. The pedestrian was struck by the driver and killed, and the driver was uninjured. No speeding, drugs, or alcohol were reported as involved.
- US 101 at MP $\mathbf{3 5 8 . 6 9}$ (north of Hoffeldt Lane): A pedestrian crash occurred in October 2020 on a foggy, wet day in the darkness. The cause was cited as the driver not yielding the right-of-way and was flagged as drugs involved. The pedestrian was struck by the driver and killed, and the driver was uninjured.
 a clear, dry day in the darkness. The pedestrian was illegally in the roadway and was struck by a vehicle; the driver of the vehicle was uninjured. No speeding, drugs, or alcohol were reported as involved.
- Winchuck River Road at MP 4.52 (east of Winriver Road): A fixed-object crash (with a tree) occurred in March 2019 on a clear, dry day at dusk. An occupant was ejected from the car and improper driving was cited as the cause. This crash was flagged as alcohol and drugs related and resulted in one fatality and one minor injury for the two people in the vehicle.
- Agness Illahe Road at MP 3.26 (south of Billings Road): A fixed-object crash (with a cut slope/ditch embankment) occurred in August 2019 on a clear, dry day in daylight. The vehicle drove too fast for the conditions and was overturned. This crash was flagged as both alcohol and drugs involved and resulted in the driver of the vehicle being killed.
- Carpenterville Highway at MP 361.63 (east of Demoss Road): An overturned crash occurred in December 2019 in the darkness. The vehicle drove too fast for the conditions and was overturned on a ditch slope or embankment. The driver was killed. No drugs or alcohol were reported as involved.

[^2]
## Collision Types

Table 4 illustrates the number and percentage of reported crashes by crash type. As shown, almost $40 \%$ of all crashes are fixed-object, representing the largest share by far of crashes. Types of fixed-objects included cut slopes or embankments, trees, guard rails, and poles. $19 \%$ of crashes were turning movement and $15 \%$ were rear-end. There were eight crashes with bicycles, of which four were categorized as turning movement, two as angle, and two as sideswipe (meeting).

Table 4. Curry County Reported Crashes by Type (2017-2021)

| Collision Type | Number of Reported Crashes | Percent of Total Crashes |
| :---: | :---: | :---: |
| Angle | 37 | $4 \%$ |
| Backing | 12 | $1 \%$ |
| Fixed-Object or Other-Object | 360 | $39 \%$ |
| Miscellaneous1 | 88 | $9 \%$ |
| Non-Collision² | 31 | $3 \%$ |
| Head-On | 9 | $1 \%$ |
| Parking Maneuver | 6 | $1 \%$ |
| Pedestrian | 22 | $2 \%$ |
| Bicyclist | 8 | $1 \%$ |
| Rear-End | 136 | $15 \%$ |
| Sideswipe (Meeting) | 22 | $2 \%$ |
| Sideswipe (Overtaking) | 25 | $3 \%$ |
| Turning Movement | 172 | $19 \%$ |
| Total | 928 | $100 \%$ |

${ }^{1}$ Typically crashes with wildlife
${ }^{2}$ Typically overturned vehicles

## Pedestrian and Bicycle Crashes

Figure 5 presents a map of pedestrian and bicycle crashes. Of the 22 pedestrian collisions, there were four crashes that resulted in a fatality (described above), four that resulted in a serious injury, and fourteen other crashes that resulted in a minor or possible injury. The serious injury crashes are detailed below:

- US 101 at MP $\mathbf{3 5 6 . 4 4}$ (north of Arnold Lane): A pedestrian crash occurred in June 2018 in cloudy, dry conditions in the daylight. The pedestrian was crossing between intersections and the driver failed to yield right-of-way to the pedestrian. The driver struck and seriously injured the pedestrian, and the driver was uninjured. No speeding, drugs, or alcohol were reported as involved.
- US 101 at MP 357.37 (east of Fern Avenue): A pedestrian crash occurred in November 2021 in rainy, wet conditions in the daylight. The pedestrian did not yield the right-of-way and was struck by the driver and seriously injured. The driver was uninjured. No speeding, drugs, or alcohol were reported as involved.
- US 101 at MP 358.35 (north of Sunshine Cove Ln): A pedestrian crash occurred in July 2018 in cloudy, wet conditions in the darkness. The pedestrian was illegally in the roadway and did not yield the right-of-way, and was reported as drinking alcohol. The driver struck and seriously injured the pedestrian, and the driver was uninjured.


UGB

- Bureau of Land Management

US Forest Service
$\square$ County Boundary

- State Border

- Bureau of Land Management

U U Forest Service
$\square$ County Boundary
_- State Border


- Memory Lane at Railroad Street: A pedestrian crash occurred in December 2018 in clear, dry conditions in the daylight. The driver failed to yield right-of-way to the pedestrian. The driver struck and seriously injured the pedestrian, and the driver was uninjured. No speeding, drugs, or alcohol were reported as involved.

Of the eight bicycle crashes, there was one crash the resulted in a fatality (described above), one that resulted in a serious injury, and six other non-fatal injury crashes. The serious injury crash is detailed below:

- US 101 at MP 336.96 (south of Myers Creek Road): A bicycle-related turning movement crash occurred in August 2019 in clear, dry conditions in the daylight. The cause was cited as the bicyclist making an improper left turn and riding in the travel lane. The bicyclist was struck and seriously injured by the vehicle. The driver of the vehicle sustained a suspected minor injury. No speeding, drugs, or alcohol were reported as involved.


## Intersection Crash Analysis

The state has identified several safety performance standards in assessing intersection safety. The intersection safety analysis evaluated the observed crash rates at the study intersections against $90^{\text {th }}$ percentile and critical crash rate thresholds, per ODOT's Analysis Procedures Manual (APM Reference 2), as well as crashes that may be occurring in excess at the study intersections. Attachment C contains the crash data worksheets as well as the intersection crash analysis worksheets.

## 90th Percentile Crash Rate

The $90^{\text {th }}$ percentile crash rate performance standard is used to identify intersections exhibiting more crashes than expected based on traffic volume. Intersection crash rates are compared to the statewide $90^{\text {th }}$ percentile crash rates for similar intersection types. Statewide $90^{\text {th }}$ percentile crash rates were developed from a study of 500 intersections in Oregon and are organized by land type and traffic control. Table 5 compares the study intersection crash rates (calculated according to ODOT APM Chapter 4) with applicable statewide $90^{\text {th }}$ percentile crash rates by intersection type.

Table 5. 90th $^{\text {th }}$ Percentile Crash Rate Comparison

| Map ID | Intersection | Total Crashes | Intersection Crash Rate ${ }^{1}$ | 90ih Percentile Crash Rate ${ }^{1}$ | Intersection rate $>90^{\text {th }}$ Rate? |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | US 101 / Floras Creek Rd | 2 | 0.23 | 0.48 | No |
| 2 | US 101 / Sixes River Rd | 1 | 0.12 | 1.08 | No |
| 3 | US 101 / Cape Blanco Hwy | 0 | 0.00 | 0.48 | No |
| 4 | US 101 / Ophir Rd | 1 | 0.20 | 0.48 | No |
| 5 | US 101 / Edson Creek Rd-Nesika Rd | 3 | 0.47 | 1.08 | No |
| 6 | US 101 / Pistol River Rd | 0 | 0.00 | 0.48 | No |
| 7 | US 101 / Cape Ferrelo Rd | 0 | 0.00 | 0.48 | No |
| 8 | US 101 / Winchuck River d | 1 | 0.05 | 1.08 | No |

IODOT APM Intersection Crash Rate per MEV equations; AADT determined using identified intersection peak hours
${ }^{2}$ ODOT APM Exhibit 4-1 for urban and rural intersections; rural rates were used for all intersections
As shown, no crashes were reported at the intersections of US 101 / Cape Blanco Highway, US 101 / Pistol River Road, and US 101 / Cape Ferrelo Road during the study period. Further, no study intersection crash rate exceeds the applicable $90^{\text {th }}$ percentile crash rate.

[^3]
## Critical Crash Rate

Critical crash rates are also calculated for the intersections based on their type and volume but require that there are sufficient reference population sites in terms of the intersection types within the group of intersections being evaluated. This method is only applicable where at least five to ten reference population sites are available for screening. Otherwise, the critical crash rate defaults to the $90^{\text {th }}$ percentile crash rates from Table 5. The study intersections include three fourleg stop-controlled and five three-leg stop-controlled intersections, therefore, a critical crash rate can be calculated for the five three-leg stop-controlled intersections and compared to their observed crash rates, as shown in Table 6. Given that there are only three four-leg stopcontrolled intersections, the observed crash rates at these intersections are compared with the $90^{\text {th }}$ percentile crash rates from Table 5. As shown, all study intersections have observed crash rates below their critical crash rates, but the US 101 / Floras Creek Road intersection is approaching its critical crash rate threshold.

Table 6. Critical Crash Rate Comparison

| Map <br> ID | Intersection | Total Crashes | Intersection <br> Crash Ratel | Critical Crash <br> Ratel | Intersection <br> Rate > Critical <br> Rate? |
| :---: | :--- | :--- | :--- | :--- | :---: |
| $\mathbf{1}$ | US 101 / Floras Creek Rd | 2 | 0.23 | 0.28 | No |
| 2 | US 101 / Sixes River Rd | 1 | 0.12 | 1.08 | No |
| 3 | US 101 / Cape Blanco Hwy | 0 | 0.00 | 0.28 | No |
| 4 | US 101 / Ophir Rd | 1 | 0.20 | 0.37 | No |
| 5 | US 101 / Edson Creek Rd-Nesika Rd | 3 | 0.47 | 1.08 | No |
| 6 | US 101 / Pistol River Rd | 0 | 0.00 | 0.30 | No |
| 7 | US 101 / Cape Ferrelo Rd | 0 | 0.00 | 0.26 | No |
| 8 | US 101 / Winchuck River d | 1 | 0.05 | 1.08 | No |

IODOT APM Intersection Crash Rate per MEV equations; AADT determine dusing identified intersection peak hours
${ }^{2}$ ODOT APM Exhibit 4-1 for urban and rural intersections; rural rates were used for all intersections

## Excess Proportion

ODOT's Excess Proportions Calculator tool was used to identify study intersections exhibiting an excess proportion of specific crash types to identify if specific changes are needed at any of the study intersections to reduce these specific crash types. The analysis showed that no intersection exhibits an excess proportion of any one crash type and the probability is low that the proportion of certain future crash types will be greater than the long-term expected proportion for the types of intersections.

## Segment Crash Analysis

All State highways and County roadways, where traffic volumes were available, were chosen for the segment crash analysis, resulting in 125 segments. Segment crash rates were calculated for each corridor according to ODOT's APM and compared to ODOT's 2022 rural highway system crash rates found in ODOT's crash rate tables (Table II). Per the APM, segment crash rates depend on the total number of crashes along the segments, segment lengths, and traffic volumes. The total number of crashes along the segments and the segment lengths were obtained from GIS data. Traffic volume data for State highways were collected from ODOT's TransGIS site, and traffic volume data for County roadways were provided by the County. Figure 6 presents the results of the analysis and Table 7 summarizes the corridors with segment crash rates that exceed the rural highway crash rates. Attachment $D$ contains the segment crash analysis worksheet.


Study Segment
Cit
BLM Land

- USFS Land

County Boundary

- State Line



Table 7. Segments with Observed Crash Rates Exceeding ODOT Rural Highway System Crash Rates

| Street | From | To | Crashes | Rural Highway System Crash Rate | Observed Crash Rate |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Oceanview Dr | Wenbourne Ln | W Benham Ln | 1 | 3.74 | 9.16 |
| Benham Ln | Oceanview Dr | Lower Harbor Rd | 4 | 1.45 | 6.14 |
| S Bank Chetco River Rd | US 101 | Harbor View Creek | 2 | 1.44 | 1.88 |
| S Bank Chetco River Rd | UGB (MP 4.0) | Eastern Terminus | 1 | 1.44 | 2.90 |
| Shopping Center Ave | W Hoffeldt Ln | Lower Harbor Rd | 9 | 1.45 | 4.17 |
| N Bank Chetco River Rd | Yellowbrick Rd | UGB (MP 5) | 7 | 1.44 | 1.92 |
| N Bank Chetco River Rd | UGB (MP 5) | Gardner Ridge Rd | 6 | 1.44 | 4.91 |
| N Bank Chetco River Rd | Gardner Ridge Rd | MP 8.5 | 5 | 1.85 | 3.12 |
| Gardner Ridge Rd | Palmer Butte | Hazel Camp Road | 3 | 1.85 | 5.80 |
| Pistol River Loop | HWY 255 | Hwy 255 (Cape View Loop) | 1 | 1.44 | 1.76 |
| Jerrys Flat Rd | UGB (MP 75.5) | Eastern Terminus | 6 | 1.22 | 2.31 |
| Agness Rd | Couger Ln | Agness Rd Fork | 1 | 1.85 | 2.36 |
| Agness Rd | Agness Rd Fork | N of Billings Rd | 3 | 1.85 | 6.84 |
| Oak Flat Rd | Agness Rd | campground road (MP 3) | 2 | 1.85 | 6.38 |
| N Bank Rogue River Rd | Cedar Valley Rd | Bluebird Ln | 4 | 1.44 | 1.60 |
| Cedar Valley Rd | N Bank Rogue River Rd | Sidney Way | 4 | 1.44 | 2.95 |
| Edson Creek Rd | N Bank Rogue River Rd | US 101 | 2 | 1.44 | 1.52 |
| Nesika Beach Rd | US 101 | Edson Creek Rd | 3 | 1.85 | 18.00 |
| Ophir Rd | US 101 | US 101 | 5 | 1.85 | 3.92 |
| Elk River Rd | US 101 | Wagner Ln | 3 | 1.44 | 1.69 |
| Elk River Rd | Wagner Ln | Haiku Ln (MP 5.5) | 2 | 1.44 | 1.50 |
| Elk River Rd/NF | Haiku Ln (MP 5.5) | County Boundary (Coos) | 1 | 1.44 | 2.33 |
| Airport Rd | US 101 | The Airport | 2 | 1.44 | 4.21 |
| Floras Lake Loop Rd | US 101 S | Us 101 N | 5 | 1.44 | 3.41 |
| Pedrioli Dr | US 101 | Western Terminus | 5 | 3.74 | 5.72 |
| W Hoffeldt Ln | South of Titus Ln | US 101 | 9 | 3.74 | 12.53 |
| Rainbow Rock Rd | Aqua Vista Ln | Carpenterville Hwy | 1 | 1.85 | 4.01 |
| Old Coast Hwy | Wedderburn Loop | US 101 | 2 | 1.85 | 13.50 |


| Street | From | To | Crashes | Rural Highway System Crash Rate | Observed Crash Rate |
| :---: | :---: | :---: | :---: | :---: | :---: |
| US 101 | Benham Ln | S Bank Chetco River Road | 46 | 1.06 | 1.17 |
| US 101 | S Bank Chetco River Road | Oak St | 40 | 1.06 | 1.91 |
| US 101 | Oak St | 5th St | 60 | 1.06 | 3.33 |
| US 101 | 5th St | Harris Park | 48 | 1.06 | 1.97 |
| US 101 | Cape Ferrelo Rd | Martin Ranch Rd | 12 | 1.22 | 1.26 |
| US 101 | Gold Beach UGB | Hunter Creek Rd | 2 | 1.06 | 1.10 |
| US 101 | Jerry's Flat Rd | N Bank Rogue River Rd | 6 | 1.06 | 1.23 |
| US 101 | Pacific Highland Dr | China Mountain Rd | 37 | 1.22 | 1.68 |
| OR 251 | US 101 | Western Terminus | 1 | 1.22 | 2.05 |
| OR 255 | US 101 N | US 101 S | 3 | 1.22 | 5.31 |
| OR 255 | Pistol River Loop | Mack Arch Rd/ US 101 | 2 | 1.22 | 6.99 |
| OR 255 | Bull Gulch Rd | Bosley Butte Rd | 1 | 1.22 | 4.67 |
| OR 255 | Whaleshead Rd | Johns PI | 3 | 1.22 | 11.00 |
| OR 255 | Johns PI | Duley Creek Rd | 9 | 1.22 | 7.18 |
| OR 255 | Duley Creek Rd | Brookings UGB | 6 | 1.22 | 5.00 |
| OR 255 | Brookings UGB | US 101 | 8 | 1.06 | 4.15 |

As shown, 44 of the 125 segments analyzed had observed crash rates that exceed the rural highway crash rates. Many of these segments exhibited one to less than one crash per year. Six segments had more than 10 total crashes over the study period, all along US 101, four of which are located in Brookings. A summary of the crash characteristics for these segments is provided in Table 8. As evinced, the majority of crashes were rear-end or turning movement, with the exception of Pacific Highland Drive to China Mountain Road; there, the vast majority of crashes were fixed-object. There were three fatal crashes along these segments (covered previously).

## Safety Priority Index System

The Safety Priority Index System (SPIS) is a methodology developed by ODOT to identify sites along State and local roads where potential safety issues warrant further investigation. The SPIS compares the number of crashes on the entire roadway, including city streets, County roads, and State highways and generates a list of sites with calculated SPIS scores. The scores are based on crash frequency, crash rate, and crash severity. SPIS sites with scores in the top 10 percent were reviewed for this crash analysis. Per the most recent SPIS list, there are no sites within the top 10 percent of crash sites. However, there are SPIS sites that are in the top 15 percent of the recent SPIS list: Carpenterville Highway at MP 358.93 and US 101 at MP 358.73. It is worth noting that there have been previous SPIS sites with scores within the top 10 percent, including US 101 from MP 317.5 to 317.45 , south of Ophir Road, and from MP 356.86 to 356.65 , at $5^{\text {th }}$ Street in Brookings.

Table 8. Crash Characteristics of Selected Segments

|  | Collision Type |  |  |  |  |  |  |  | Severity |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment | Angle | FixedObject | Non-collision /Miscellaneous | Pedestrian | Rear-End | Sideswipeovertaking | Sideswipemeeting | Turning Movement | Fatal | Non-Fatal Injury | PDO | Total |
| US 101 from Benham In to S Bank Chetco River Road | 1 | 3 | - | 8 | 10 | 4 | 1 | 19 | 2 | 31 | 13 | 46 |
| US 101 from S Bank Chetco River Road to Oak St | - | 2 | 1 | 3 | 15 | 1 | - | 18 | 1 | 22 | 17 | 40 |
| US 101 from Oak St to $5^{\text {th }} \mathbf{S t}$ | 7 | 3 | 2 | 3 | 21 | 7 | - | 17 | 0 | 37 | 23 | 60 |
| $\begin{aligned} & \text { US } 101 \text { from 5ih St } \\ & \text { to Harris Park } \end{aligned}$ | 1 | 5 | 2 | 1 | 18 | 1 | 1 | 19 | 0 | 29 | 19 | 48 |
| US 101 from Cape Ferrelo Rd to Martin Ranch Rd | - | 1 | 5 | - | 3 | - | 2 | 1 | 0 | 5 | 7 | 12 |
| US 101 from Pacific Highland Dr to China Mountain Rd | - | 26 | 8 | - | - | - | - | 3 | 0 | 15 | 22 | 37 |

[^4]
## Crash Analysis Findings

- 928 crashes were reported in Curry County between 2017 and 2021.
- $59 \%$ of all reported crashes resulted in some level of injury, including 14 fatal crashes and 45 serious injury crashes ( 4 of the fatal crashes included pedestrians and included a bicyclist).
$39 \%$ of all reported crashes were with a fixed or other object, $19 \%$ were turning movement, and $15 \%$ were rear-end.
- 22 of the reported crashes included pedestrians and 8 included bicyclists.
- No crashes were reported at the US 101 / Cape Blanco Highway, US 101 / Pistol River Road, or US 101 / Cape Ferrelo Road intersections during the study period.
- No study intersection has an observed crash rate that exceeds the applicable 90th percentile crash rate or critical crash rate (the US 101 / Floras Creek Road intersection is approaching its critical crash rate threshold).
- No study intersection exhibits an excess proportion of any one crash type.
- 44 of the 125 study segments have observed crash rates that exceed the rural highway crash rates (many exhibited less than one crash per year).
- 6 segments, all along US 101, had more than ten total crashes, 4 of which are in Brookings (most the crashes were rear-end or turning movement and included 3 of the fatal crashes).
- There are no SPIS sites in the top $10 \%$ within Curry County from the most recent SPIS list.


## FREIGHT ANALYSIS

The freight analysis presented in this section identifies potential issues with current freight movements within Curry County, including freight route restrictions, over-dimensional load pinch points, bridges, and other mobility issues and freight needs identified in the Oregon Freight Plan (OFP - Reference 3). As described in Technical Memorandum \#3 (Update System Inventory), the Oregon Highway Plan (OHP - Reference 4) does not designate any of the State highways in Curry County as freight routes, including US 101, but the OHP does designate US 101 as a Reduction Review Route. Although no State highway in the county is designated as a freight route, the freight analysis focuses on the State highway system as it generally experiences the highest percentage of heavy vehicle traffic within the county, and therefore, needs to be able to accommodate efficient freight truck movements.

## Freight Route Restrictions

From Technical Memorandum \#3 (Update System Inventory), ODOT's Motor Carrier Transportation Division (MCTD) identifies State highway freight restrictions, in terms of overdimensional loads, on various State route maps. All MCTD maps were reviewed to determine potential issues with freight truck movements in Curry County.

Relevant State highway freight restrictions include:

- US 101
- Maximum Allowable Length: Truck-Tractor and Semitrailer - 60 feet overall (53-foot trailer)
- Overall Length Limit: Pickup Truck and Trailer - 70 feet overall (53-foot trailer)
- Triples Combinations: Not Authorized
- Weight Restricted Bridges: Reinhart Creek (MP 311.40) south of Port Orford and Connector Road (MP 326.47) in Gold Beach
- OR 250 (Cape Blanco Highway) and OR 255 (Carpenterville Highway)
- Maximum Allowable Length: Truck-Tractor and Semitrailer - 60 feet overall (40-foot trailer)
- Vehicle Widths Exceeding 12 Feet: Not Approved for Continuous Operation
- Triples Combinations: Not Authorized
- Continuous Movement of 14 Feet Wide Mobile Homes / Modular Building Units: Not Authorized
- Loads not Wider than 14 Feet (if gross weight does not exceed 98,000 pounds): Not Authorized


## Alternative Routes

In cases where freight need to access and/or travel through Curry County along other routes than the primary arterials, US 101, US 199 (via California), or OR 42 (via Coos County), potential alternative routes are limited to the following:

- US 101 Alternative Routes (North-South)
- Cedar Valley Road
- Edson Creek Road
- N Bank Rogue River Road
- OR 255 (Carpenterville Highway)
- Pistol River Loop Road
- Ocean View Drive
- US 199 and OR 42 Alternative Routes (East-West)
- N Bank Rogue River Road
- Jerry's Flat Road
- Agness Road
- Galice Creek Road

In their current state, these roadways may not be appropriate for moving freight, but could be considered for local freight route designations.

## Freight Pinch Points and Bottlenecks

ODOT produced a Highway Over-Dimensional Load Pinch Points study in 2016 that identifies highway pinch points restricting movement of over-dimensional loads. The study is intended to help implement OFP strategies that can improve the efficiency of these movements. Overdimensional load pinch points are caused by height, width, weight, or length constraints (as indicated in the previous section), and can include low overpasses, narrow roadways or intersections, sharp curves, weight-restricted bridges, bridges with low overhead clearance, sign bridges, tunnels, and other characteristics.

The study resulted in 381 pinch points across the state and classified 92 of them as high-priority based on the degree to which resolving a pinch point would open up an entire corridor for overdimensional loads. Based on Appendix H of the OFP, no State highway in Curry County has any high-priority pinch points. However, a low-priority pinch point was identified on US 101 south of Port Orford. ODOT provides the following details on this pinch point:

- Region 3, District 7 Pinch Point \#29 (Wide Load)
- Location: US 101 near Humbug Mountain State Park from MP 306.23 to 308.03 (approximately 1.8 miles), in both travel directions
- Approximate number of Over-Dimensional Loads/month: 33
- Description: Roadway has sharp curves and narrow shoulders that winds around the state park / large rock faces. Pinch point removal would involve significant environmental constraints and major earthwork and construction.

The OFP also identifies the top 200 miles of freight bottlenecks across the state, which are typically caused by traffic congestion and result in stop-and-go traffic flow and long backups. They can also result from nonrecurring congestion such as construction work zones, traffic incidents (e.g., crashes and breakdowns), extreme weather conditions (e.g., landslides), and substandard traffic control. The top 200 miles of bottlenecks in the state are mostly concentrated around the Portland Metro area, therefore, no bottlenecks are identified on the State highway system in Curry County.
Lastly, the OFP also identifies high-priority seismic landslide locations, specific freight impacts on State highways, and freight highway delay areas, but none are located on the State highway system in Curry County.

## Bridges

From Technical Memorandum \#3 (Update System Inventory), 29 bridges in the county are either identified as structurally deficient, weight restricted, or having a sufficiency rating below 50. These bridges are summarized in Table 9.

- Structurally deficient (SD) bridges have a deck, superstructure, or substructure, that is rated as being in poor or worse condition.
- Weight restricted (WR) bridges or bridges posted for load (LP) may not have the capacity to safely carry certain loads / load combinations, requiring some freight use alternate routes, if available.
- Sufficiency Ratings (SR) represent a bridge's sufficiency to remain in service, based on several factors, and range from zero (entirely insufficient) to 100 (entirely sufficient). A sufficiency rating below 50 makes a bridge eligible for replacement.

Table 9. Structurally Deficient or Weight Restricted Bridges or Bridges with Sufficiency Ratings Below 50

| Name | ID | Owner | Carries | Crosses | MP | SD | WR/LP | SR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mill Creek | 15C23 | County | S Bank Chetco River Rd | Mill Creek | 5.70 |  | $\checkmark$ | N/A |
| Pistol River | 15C33 | County | Pistol River Loop Rd (Co. Rd 693) | Pistol River | 0.30 | $\checkmark$ |  | 30.1 |
| Hunters Creek | 15C24 | County | Hunter Creek Rd | Hunters Creek | 0.90 |  |  | 44.3 |
| Hunter Creek | 15C010 | County | Hunter Creek Rd | Hunter Creek | 0.20 |  | $\checkmark$ | 37.7 |
| Rogue River | 01172 | ODOT | US 101 | Rogue River | 327.70 |  |  | 37.4 |
| Hwy 9 over Conn Rd | 16014 | ODOT | US 101 | Ocean Way (Conn. Rd) | 326.47 |  | $\checkmark$ | N/A |
| Euchre Creek | 15C31 | County | Ophir Rd (Co. <br> Road 510) | Euchre Creek | 0.10 |  | $\checkmark$ | 22.9 |
| Reinhart Creek | 07514 | ODOT | US 101 | Reinhart Creek | 311.40 |  | $\checkmark$ | N/A |
| Myrtle Creek | 15C15 | County | Arizona Ranch Rd (Co. Road 500) | Myrtle Creek | 0.30 |  | $\checkmark$ | 34.3 |
| Arizona Beach | 20962 | State Park | Pedestrian Bridge | Myrtle Creek | 0.00 | $\checkmark$ |  | Unknown |
| Bear Trap Creek | 02386A | ODOT | US 101 | Bear Trap Creek | 308.84 |  |  | 47.5 |
| Brush Creek Trail (\#2) | 21516 | State Park | Bike/ Pedestrian | Brush Creek | 0.00 | $\checkmark$ |  | Unknown |
| Brush Creek Trail (\#1) | 21514 | State Park | Bike/ Pedestrian | Creek | 0.00 | $\checkmark$ |  | Unknown |
| Humbug Mt Trail Bridge | 21518 | State Park | Bike/ Pedestrian | Trail | 0.00 | $\checkmark$ |  | Unknown |
| Edson Creek | 15C004 | County | Sixes River Rd | Edson Creek | 4.20 |  | $\checkmark$ | N/A |
| North Floras Creek | 15C26 | County | Floras Creek Rd | Floras Creek ( N Fork) | 8.90 | $\checkmark$ |  | 17.1 |
| US 101 Sign Cantilever (SB) | 22369 | Private (NonRailroad) | Sign Cantilever | US 101 | 354.97 |  |  | Unknown |
| US 101 Sign Cantilever (NB) | 22373 | Private (NonRailroad) | Sign Cantilever | US 101 | 355.27 |  |  | Unknown |
| Humbug Day Use Area | 19783 | State Park | Park Rd | Brush Creek | 307.02 |  |  | Unknown |
| Humbug Mt. Pedestrian Bridge | 21005 | State Park | Park Rd | Brush Creek | 0.00 |  |  | Unknown |
| Brush Creek Trail Bridge No. 3 (South End) | 21515 | State Park | Bike/Pedestrian | Brush Creek | 0.00 |  |  | Unknown |
| Ped Brush Creek Humbug Mtn Park Trail | 22713 | State Park | Humbug Mtn Trail | Brush Creek | 0.00 |  |  | Unknown |
| Brush Creek | 18096 | ODOT | US 101 | Brush Creek | 306.35 |  |  | 29.8 |
| Winchuck River | 09091A | ODOT | US 101 | Winchuck River | 362.61 |  |  | 42.0 |
| N Fork Chetco River | 15C14 | County | N Bank Chetco River Rd | N Fork Chetco River | 5.30 |  |  | 43.1 |
| Willow Creek | 15C12 | County | Co. Road 136 | Willow Creek <br> (EB) | 0.40 |  |  | 44.8 |
| Floras Creek | 09370 | ODOT | US 101 | Floras Creek | 288.50 |  |  | 46.3 |
| Elk River | 00902C | ODOT | US 101 | Elk River | 297.37 |  |  | 46.4 |
| Morton Creek | 00912 | ODOT | US 101 | Morton Creek | 286.61 |  |  | 48.9 |

[^5]In addition to these freight limitations, the OFP identifies seismic bridges across the state, but none are located on the State highway system in Curry County.

## Intermodal Connector Roads

Intermodal connectors are the links that facilitate transfers between modes, such as local roads between a designated freight route and a port or rail reload facility. The OFP identifies these routes across the state and any conditions they have that could impact freight movement. The OFP includes these two intermodal connector roads in Curry County and their needs:

- Lower Harbor Road (Brookings): congestion, safety, signage, mixing with traffic
- Dock Road to Harbor Drive (Port Orford): pavement condition, roadway width, parking, striping, signage, turning movements, mixing with pedestrians


## Freight Analysis Findings

- US 101, OR 250, and OR 255 are restricted to moving freight up to specific lengths and widths (more details are available on MCTD route maps) and are not authorized to move triples combinations.
- US 101 includes weight restricted bridges across Reinhart Creek (MP 311.40) south of Port Orford and Connector Road (MP 326.47) in Gold Beach.
- OR 250 and OR 255 are not authorized for continuous movement of 14 feet wide mobile homes / modular building units.
- Alternative freight routes to the primary arterials accessing Curry County - US 101, US 199, OR 42 - are limited and may not be appropriate for moving freight in their current state.
- One low-priority freight pinch point is located on US 101 that is approximately 1.8 miles long near Humbug Mountain State Park. Removing the pinch point would involve significant environmental constraints and major earthwork and construction.
- No high-priority bottlenecks, seismic landslide sites, specific freight impacts, or freight highway delay areas are identified on the State highway system in the county.
- 29 bridges are identified as either being structurally deficient (6), weight restricted (7), or having sufficiency ratings below 50 (25); one bridge can be all three of these.
- 10 of these bridges are under County jurisdiction and the remaining are owned by ODOT, the State Parks, or privately held.
- No seismic bridges are identified on the State highway system in Curry County.
- Lower Harbor Road in Brookings and Dock Road to Harbor Drive in Port Orford are intermodal connectors with identified needs for moving freight.


## MULTIMODAL ANALYSIS

A review of multimodal conditions across Curry County that is presented in the following sections included an assessment of traffic stress that people experience biking, the quality of facilities for people walking and using transit, and the safety risk of the current transportation system to people walking, rolling, and biking.

## Bicycle Level of Traffic Stress

ODOT's Bicycle Level of Traffic Stress (BLTS) methodology uses four scores to describe and evaluate the stress that a person can experience while biking on a roadway. These scores range from BLTS 1 (little traffic stress) to BLTS 4 (high traffic stress) and depend on numerous characteristics of biking facilities. BLTS is determined based on roadway characteristics such as vehicular speed, number of travel lanes per direction, the presence and width of on-street bicycle facilities and/or adjacent parking lane, the presence and width of paved shoulders, etc. Table 10 defines each BLTS score. Per ODOT, BLTS 2 is often used as a target for most roadways as it will typically appeal to most of the potential bike-riding population and maximize the available bicycle mode share.

Table 10. Bicycle Level of Traffic Stress (BLTS) Scores

| $\begin{aligned} & \text { BLTS } \\ & \text { Scores } \end{aligned}$ | Definition of BLTS Segment, Suitability, and Condition |
| :---: | :---: |
| 1 | Represents little to no traffic stress, suitable for all cyclists including children who are trained to safely cross intersections alone and children supervised by parents. Traffic speeds and volumes are low. Typically includes residential local streets and separated bike paths/cycle tracks. |
| 2 | Represents little traffic stress but requires more attention than what young children can handle, so is suitable for teen and adult cyclists with bike handling skills. Traffic speeds and volumes are slightly higher than BLTS 1 streets, but speed differentials are still low. Typically includes collector-level streets with bike lanes or a central business district. |
| 3 | Represents moderate stress and is suitable for most observant adult cyclists. Traffic speeds and volumes are moderate. Typically includes low-speed arterials with bike lanes or moderate speed non-multilane roadways. |
| 4 | Represents high traffic stress and suitable for experienced and skilled cyclists. Traffic speeds and volumes are moderate to high. Typically includes high-speed or multilane roadways with narrow or now bike lanes. |

Figure 7 illustrates the results of the BLTS analysis, which was only conducted for those arterials and collectors within the county where traffic volume data were available. Additionally, the BLTS along State highways - US 101, OR 250, OR 251, and OR 255 - are produced and provided by ODOT. As shown, most roadways score with a BLTS 2 or 3 and only few roadways or sections of roadway score with a BLTS 1 or BLTS 4. The BLTS scoring of roadways generally lowers further from the urbanized areas of the county where daily traffic volumes decrease and the roadway environment becomes more rural. Even without dedicated biking facilities, rural roadways can receive a score of BLTS 2 if bicyclists are sharing the roadway with fewer vehicles, depending on the posted or prevailing speed. The lack of dedicated bicycle facilities may still be uncomfortable for some riders, despite a BLTS score of 2.

Conversely, most roadways with higher BLTS scores are generally centered around the urbanized areas of the county and are attributed to higher traffic volumes and higher posted speeds, especially without dedicated biking facilities.

Attachment E includes the BLTS analysis worksheet.

## Pedestrian and Transit Qualitative Multimodal Assessment

ODOT's Qualitative Multimodal Assessment (QMA) methodology evaluates multimodal facilities and services for people walking and taking transit in primarily rural environments to identify areas where improvements might be needed for these users. The QMA applies subjective ratings (e.g., Excellent, Good, Fair, Poor) to arterials and collectors throughout the county based on available facilities and services.



Bicycle Level of Traffic Stress


## Pedestrian QMA

The following six criteria are used to assess the quality of facilities for people walking and rolling, which were applied to the arterials and collectors in Curry County:

- Outside travel lane width
- Bicycle lane/shoulder width
- Number of travel lanes
- Posted speed
- Traffic volumes
- Presence of buffers (landscaped or other)
- Presence and width of sidewalk/paths
- Presence and scale of lighting

Figure 8 illustrates the results of the Pedestrian QMA. Under existing conditions, a majority of arterials and collectors result in "Poor" Pedestrian QMA ratings, except within and near the incorporated cities and unincorporated communities. These "Poor" ratings along County facilities are generally due to a lack of walking facilities, including paved shoulders, lack of lighting, and where there are high posted speeds. US 101 is primarily rated as "Fair" near the incorporated cities and unincorporated communities but does demonstrate "Good" ratings for short sections of highway within Port Orford and Brookings, due to presence of sidewalks, lighting, low posted speeds, and presence of buffers in some cases. US 101 and other State highways are rated as "Poor" generally due to outside travel lane widths, number of travel lanes, lack of paved shoulder, lack of lighting, and high traffic volumes.

Attachment Fincludes the Pedestrian QMA worksheet.

## Transit QMA

The following four criteria are used to assess the quality of facilities and services for people taking transit, which were applied to the available fixed-route transit services in Curry County, Curry Public Transit's (CPT) Coastal Express and the POINT's SouthWest route:

- Frequency and on-time reliability
- Schedule speed/travel times
- Transit stop amenities
- Connecting pedestrian/bicycle network

The CPT Coastal Express provides the following services and facilities within the county:

- Peak hour morning, mid-day, and evening loop service between Harbor and Port Orford, with stops in between and destinations beyond in Smith River and North Bend.
- One loop each during the AM, mid-day, and PM peak.
- Headways during all hours of operation are greater than one hour.
- Bus stops, official and flag, are located in Harbor, Brookings, Gold Beach, Port Orford, and Langlois with amenities that vary from covered shelters to no signs or waiting areas.



| Pedestrian QMA Rating | W/I// City Boundaries |
| :---: | :---: |
| - Good |  |
| - Fair | Bureau of Land Management |
| - Poor | - US Forest Service |
|  | $\square$ County Boundary |

Additional details on CPT Coastal Express bus stops in Curry County are summarized in Table 11, including the presence and quality of bus stop amenities. As shown, amenities are limited or not present at some stops and many stops in the CPT system lack proper signage. Further, some stops connect to sidewalks, bike lanes, and/or crossings, but others do not.

Table 11. CPT Bus Stop Amenities

| Stop | Amenities | Walking Access | Biking Access | Notes |
| :---: | :---: | :---: | :---: | :---: |
| Langlois Public Library (Flag Słop) | - No amenities | Poor | Poor | - Proposed to be a designated CPT bus stop <br> - No sidewalks present on US 101 or Waller Lane <br> - No bike lanes in the vicinity <br> - No crossings present |
| Langlois Store (Flag Stop) | - No amenities | Poor | Fair | - No sidewalks present on US 101 <br> - Bike lane present on US 101 (southbound) <br> - No crossings present |
| Ray's Food Place, Port Orford | - Covered shelter and waiting area <br> - Bench <br> - Trash receptacle | Fair | Good | - Sidewalks / bike lanes located along US 101 <br> - No US 101 crossing opportunities near bus stop <br> - Stop located in parking lot |
| Ray's Food Place, Gold Beach | - CPT bus stop sign <br> - Covered shelter and waiting area <br> - Bench <br> - Street lighting | Good | Poor | - Sidewalks connect to stop <br> - Protected crossings available at US $101 / 6$ th Street <br> - No bike lanes present <br> - Stop located in parking lot |
| $5^{\text {th }}$ Street/ Bankus Park, Brookings | - CPT bus stop sign <br> - Covered shelter and waiting area <br> - Bench <br> - Bike racks | Fair | Fair | - Sidewalk network not well connected (no sidewalk on north leg of Pacific Avenue) <br> - Curb ramps present (not up to ADA standards) <br> - SouthWest POINT bus stop in vicinity (not near covered waiting area) <br> - Bike lanes present along US 101 and 5th St <br> - Protected crossings present at US $101 / 5$ th St <br> - Two direct pedestrian access points are available from sidewalk along US 101 <br> - Stop located in parking lot |
| Chevron Station, Harbor | - Temporary CPT bus stop sign | Fair | Fair | - Sidewalks / bike lanes located along US 101 <br> - Sidewalk gaps / no bike lanes along Zimmerman Lane and Hoffeldt Lane <br> - Protected crossings at US 101/Zimmerman Lane (north of stop) and at US 101/W Hoffeldt Lane (south of stop) <br> - An on-street parking area for pull-outs is being used by trucks |
| McKay's Market, Harbor | - No amenities | Fair | Fair | - Sidewalks / bike lanes located along US 101 <br> - Sidewalk gaps / no bike lanes along Zimmerman Lane and Hoffeldt Lane <br> - Protected crossings at US 101/Zimmerman Lane (north of stop) and at US 101/W Hoffeldt Lane (south of stop) <br> - Stop located in parking lot (no indication of a bus stop) |

Walking and Biking Rating: Good = sidewalks and crosswalks; bicycle lanes or shared lane markings; Fair = some sidewalks; adequate shoulder for biking; Poor = no facilities

Southwest POINT provides mid-day loop service between Brookings and Cave Junction (and beyond to Klamath Falls via connecting services). There is one eastbound bus and one
westbound bus, both stopping in Brookings at a stop with a covered shelter and access to sidewalks, bike lanes, and a crossing of US 101. Based on these services and facilities, the Transit QMA rating for the county is "Fair." Although service is rated as "Fair," it is important to note that available services and facilities are currently rural and provide countywide coverage, and more. Attachment G includes the Transit QMA worksheet.

## Statewide Facility Safety Risk Assessment

The statewide bicycle and pedestrian safety risk assessment focuses on the safety of people walking, rolling, and biking along State highways (US 101, OR 250, OR 251, and OR 255 within Curry County) and their risk of being involved in crashes. The State of Oregon has identified the following factors to assess safety risk of its highways:

- Roadway Classification
- Number of Lanes
- Access Density
- Presence of Sidewalks/Bike Lanes
- Posted Speed
- Zoning
- Proximity to Schools
- Proximity to Transit Stops
- Population over Age 64

These factors were established through analyzing crash, traffic, infrastructure, land use, and demographic data across the State of Oregon. A weight is assigned to each factor based on its correlation to crash history: factors with higher weights have stronger correlations with severe crashes. Other characteristics not listed above that ODOT recommends should be investigated at intersections include high turning volumes, left-turn signal phasing (traffic signals), lighting, propensity for mid-block crossings, and exposure to traffic volumes.
The application of risk factors was completed by ODOT on a statewide level, and therefore, highway segments are grouped to show how one segment might compare to others in Oregon. While these groupings highlight general safety needs along the State highways in Curry County, they can also help with prioritizing improvements where safety risk may be higher in some State highway segments than in others.
Figure 9 and Figure 10 show the varying levels of safety risk for bicyclists and pedestrians on the State highway system in the county. The greatest safety risks to bicyclists on the State highway system in the county are identified along US 101. Bicycle safety risk is relatively high in the corridor throughout the county but is highest within the Brookings UGB likely due to a mixture of all the factors listed previously.


Bicycle Safety Risk Curry County, Oregon


Bicycle Safety Risk Curry County, Oregon


Bicycle Safety Risk Curry County, Oregon



Pedestrian Safety Risk


Pedestrian Safety Risk

The greatest safety risks to pedestrians on the State highway system in the county are also identified along US 101 likely due to its 'principal arterial' classification, as well as other factors in the following areas:

- Near Airport Road (adjacent to Pacific High School, posted speed, demographics)
- Within Port Orford city limits (access density, number of lanes, proximity to transit, demographics, zoning)
- Within the northern Gold Beach city limits (access density, number of lanes, proximity to Gold Beach High School and transit, and lack of sidewalks south of Kerber Dr)
- Cape Sebastian area north of Pistol River (demographics, posted speed, number of lanes, zoning)
- Within the central part of the Brookings UGB (demographics, number of lanes, access density, proximity to schools and transit)


## Multimodal Analysis Findings

- Most arterials and collectors, or sections of these roadways, score with a BLTS 2 or 3. Roadway BLTS scores generally lower further from the urbanized areas of the county, where traffic volumes are lower, and higher in the urbanized areas, depending on traffic volume levels, posted speeds, and availability of dedicated biking facilities.
- Most arterials and collectors result in "Poor" Pedestrian QMA ratings - except within and near the incorporated cities and unincorporated communities. This is generally due to a lack of walking facilities and lighting and where posted speeds are high.
- US 101 is primarily rated as "Fair" near the incorporated cities and unincorporated communities and demonstrates some "Good" ratings in Port Orford and Brookings.
- Some sections of US 101 and the remaining State highways are rated as "Poor."
- Transit services and facilities in the county results in a Transit QMA rating of "Fair" primarily due to its frequency.
- Safety risks to bicyclists are relatively high along the US 101 corridor throughout the county but is highest within the Brookings UGB. This is due to factors such as access density, proximity to transit and schools, demographics, and zoning of adjacent lands.
- The greatest safety risks to pedestrians are the highest on US 101 near Airport Road, within the city limits of Port Orford and Gold Beach (northern) and the Brookings UGB, and near the Cape Sebastian area north of Pistol River.


## REFERENCES

1. Transportation Research Board. Highway Capacity Manual, $7^{\text {th }}$ Edition, 2022.
2. Oregon Department of Transportation. Analysis Procedures Manual, 2022.
3. Oregon Department of Transportation. Oregon Freight Plan, Draft - Anticipated March 2023.
4. Oregon Department of Transportation. Oregon Highway Plan, 2015.

## ATTACHMENT A - TRAFFIC COUNT WORKSHEETS



Comments:


Quality Counts
DATA THAT DRNES COMMUNITES


| 5-Min Count Period Beginning At | US 101 <br> (Northbound) |  |  |  | US 101 <br> (Southbound) |  |  |  | Pistol River Rd/Carpenterville Rd Connector (Eastbound) |  |  |  | Pistol River Rd/Carpenterville Rd Connector (Westbound) |  |  |  | Total | Hourly Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |
| 4:00 PM | 0 | 10 | 0 | 0 | 1 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 26 |  |
| 4:05 PM | 0 | 12 | 2 | 0 | 0 | 19 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 35 |  |
| 4:10 PM | 0 | 12 | 0 | 0 | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 35 |  |
| 4:15 PM | 0 | 12 | 0 | 0 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 29 |  |
| 4:20 PM | 0 | 20 | 0 | 0 | 0 | 17 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 39 |  |
| 4:25 PM | 0 | 19 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 27 |  |
| 4:30 PM | 0 | 12 | 1 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 24 |  |
| 4:35 PM | 0 | 12 | 0 | 0 | 0 | 20 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 34 |  |
| 4:40 PM | 0 | 19 | 0 | 0 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 36 |  |
| 4:45 PM | 0 | 16 | 2 | 0 | 1 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 41 |  |
| 4:50 PM | 0 | 8 | 0 | 0 | 1 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 18 |  |
| 4:55 PM | 0 | 7 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 358 |
| 5:00 PM | 0 | 12 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 | 354 |
| 5:05 PM | 0 | 5 | 0 | 0 | 1 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 17 | 336 |
| 5:10 PM | 0 | 9 | 0 | 0 | 0 | 13 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 324 |
| 5:15 PM | 0 | 16 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 27 | 322 |
| 5:20 PM | 0 | 9 | 1 | 0 | 0 | 18 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 29 | 312 |
| 5:25 PM | 0 | 14 | 0 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 32 | 317 |
| 5:30 PM | 0 | 12 | 0 | 0 | 0 | 23 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 35 | 328 |
| 5:35 PM | 0 | 11 | 0 | 0 | 2 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 28 | 322 |
| 5:40 PM | 0 | 12 | 1 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 29 | 315 |
| 5:45 PM | 0 | 15 | 2 | 0 | 1 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 27 | 301 |
| 5:50 PM | 0 | 5 | 2 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 20 | 303 |
| 5:55 PM | 0 | 11 | 3 | 0 | 1 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 26 | 315 |
| Peak 15-Min Flowrates | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  | Westbound |  |  |  | Total |  |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |  |
| All Vehicles | 0 | 188 | 8 | 0 | 4 | 232 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 8 | 0 |  | 44 |
| Heavy Trucks Buses | 0 | 28 | 0 |  | 0 | 32 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  |  | 0 |
| Pedestrians |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  | 0 |
| Bicycles Scooters | 0 | 4 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  |  | 4 |

Comments:

Location: US 101 -- Winchuck River Rd/Oceanview Dr
Date: 9/28/2022
Site Code: 15952903

|  | $\begin{gathered} \hline \text { US } 101 \\ \text { Southbound } \end{gathered}$ |  |  |  | Winchuck River Rd Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Start } \\ & \text { Time } \\ & \hline \end{aligned}$ | Right | Thru | Left | U-Turn | $\begin{gathered} \text { Right Slip } \\ \text { Lane } \end{gathered}$ | Right | Thru | Left | U-Turn |
| 04:00 PM | 0 | 30 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| 04:05 PM | 0 | 41 | 0 | 0 | 2 | 0 | 0 | 0 | 0 |
| 04:10 PM | 1 | 40 | 4 | 0 | 5 | 0 | 0 | 2 | 0 |
| 04:15 PM | 1 | 36 | 1 | 0 | 5 | 0 | 1 | 0 | 0 |
| 04:20 PM | 1 | 30 | 2 | 0 | 3 | 0 | 1 | 0 | 0 |
| 04:25 PM | 2 | 36 | 4 | 0 | 2 | 0 | 0 | 0 | 0 |
| 04:30 PM | 2 | 25 | 4 | 0 | 5 | 1 | 0 | 0 | 0 |
| 04:35 PM | 3 | 27 | 5 | 0 | 5 | 0 | 0 | 0 | 0 |
| 04:40 PM | 1 | 37 | 4 | 0 | 0 | 0 | 0 | 0 | 0 |
| 04:45 PM | 1 | 32 | 1 | 1 | 0 | 0 | 0 | 0 | 1 |
| 04:50 PM | 2 | 23 | 6 | 0 | 3 | 0 | 0 | 1 | 0 |
| 04:55 PM | 4 | 37 | 5 | 0 | 4 | 0 | 0 | 2 | 0 |
| 05:00 PM | 0 | 28 | 3 | 0 | 1 | 0 | 0 | 0 | 0 |
| 05:05 PM | 2 | 29 | 5 | 0 | 1 | 0 | 1 | 2 | 0 |
| 05:10 PM | 0 | 38 | 2 | 0 | 3 | 1 | 0 | 1 | 0 |
| 05:15 PM | 5 | 42 | 3 | 0 | 2 | 0 | 0 | 0 | 0 |
| 05:20 PM | 1 | 31 | 3 | 0 | 1 | 0 | 0 | 0 | 0 |
| 05:25 PM | 1 | 26 | 5 | 0 | 0 | 0 | 1 | 0 | 0 |
| 05:30 PM | 2 | 25 | 3 | 0 | 1 | 0 | 0 | 0 | 0 |
| 05:35 PM | 2 | 30 | 9 | 0 | 3 | 0 | 0 | 0 | 0 |
| 05:40 PM | 0 | 31 | 4 | 0 | 2 | 0 | 0 | 0 | 0 |
| 05:45 PM | 0 | 21 | 3 | 0 | 2 | 0 | 0 | 0 | 0 |
| 05:50 PM | 0 | 13 | 3 | 0 | 3 | 0 | 0 | 1 | 0 |
| 05:55 PM | 1 | 17 | 1 | 0 | 4 | 0 | 1 | 1 | 0 |
| Total | 32 | 725 | 80 | 1 | 58 | 2 | 5 | 10 | 1 |

Peak Hour: 4:00 PM - 5:00 PM
Peak 15: 4:05 PM - 4:20 PM
PHF: 0.922

| US 101 Northbound |  |  |  | Oceanview Dr Eastbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Right | Thru | Left | U-Turn | Right | Thru | Left | U-Turn |
| 3 | 38 | 3 | 0 | 1 | 0 | 1 | 0 |
| 1 | 34 | 0 | 0 | 1 | 0 | 0 | 0 |
| 1 | 31 | 3 | 0 | 1 | 0 | 2 | 0 |
| 0 | 31 | 3 | 0 | 2 | 0 | 1 | 0 |
| 0 | 34 | 2 | 0 | 3 | 1 | 1 | 0 |
| 2 | 30 | 1 | 0 | 1 | 0 | 3 | 0 |
| 0 | 21 | 1 | 0 | 1 | 0 | 1 | 0 |
| 1 | 34 | 2 | 0 | 1 | 0 | 0 | 0 |
| 0 | 32 | 4 | 0 | 2 | 0 | 1 | 0 |
| 0 | 31 | 1 | 0 | 2 | 0 | 1 | 1 |
| 2 | 28 | 2 | 0 | 1 | 0 | 0 | 0 |
| 0 | 22 | 1 | 0 | 0 | 0 | 1 | 0 |
| 1 | 32 | 1 | 0 | 1 | 0 | 1 | 0 |
| 0 | 29 | 2 | 0 | 1 | 0 | 0 | 0 |
| 0 | 26 | 2 | 0 | 1 | 2 | 0 | 0 |
| 0 | 24 | 1 | 0 | 0 | 3 | 0 | 0 |
| 0 | 29 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0 | 24 | 2 | 0 | 0 | 0 | 0 | 0 |
| 0 | 34 | 1 | 0 | 2 | 0 | 0 | 0 |
| 0 | 8 | 1 | 0 | 1 | 0 | 0 | 0 |
| 0 | 26 | 2 | 0 | 2 | 0 | 0 | 0 |
| 0 | 17 | 1 | 0 | 0 | 1 | 0 | 0 |
| 0 | 17 | 3 | 0 | 0 | 0 | 0 | 0 |
| 1 | 30 | 1 | 0 | 0 | 0 | 1 | 0 |
| 12 | 662 | 40 | 0 | 24 | 7 | 14 | 1 |



Comments:


Comments:


Peak-Hour: 4:15 PM -- 5:15 PM
Peak 15-Min: 4:25 PM -- 4:40 PM


| 5-Min Count Period Beginning At | US 101 <br> (Northbound) |  |  |  | US 101 (Southbound) |  |  |  | Ophir Rd/Euchre Creek Rd Connector (Eastbound) |  |  |  | Ophir Rd/Euchre Creek Rd Connector (Westbound) |  |  |  | Total | Hourly Totals |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |
| 4:00 PM | 0 | 9 | 0 | 0 | 1 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 20 |  |
| 4:05 PM | 0 | 10 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 16 |  |
| 4:10 PM | 0 | 6 | 1 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 18 |  |
| 4:15 PM | 0 | 11 | 0 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 |  |
| 4:20 PM | 0 | 4 | 0 | 0 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 17 |  |
| 4:25 PM | 0 | 6 | 0 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 22 |  |
| 4:30 PM | 0 | 6 | 0 | 0 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 |  |
| 4:35 PM | 0 | 11 | 0 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 25 |  |
| 4:40 PM | 0 | 6 | 0 | 0 | 1 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 18 |  |
| 4:45 PM | 0 | 7 | 0 | 0 | 1 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 20 |  |
| 4:50 PM | 0 | 8 | 0 | 0 | 1 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 17 |  |
| 4:55 PM | 0 | 3 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 14 | 231 |
| 5:00 PM | 0 | 4 | 0 | 0 | 1 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 228 |
| 5:05 PM | 0 | 3 | 3 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 18 | 230 |
| 5:10 PM | 0 | 12 | 1 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 27 | 239 |
| 5:15 PM | 0 | 11 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 20 | 237 |
| 5:20 PM | 0 | 5 | 0 | 0 | 1 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 234 |
| 5:25 PM | 0 | 7 | 0 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 18 | 230 |
| 5:30 PM | 0 | 3 | 1 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 219 |
| 5:35 PM | 0 | 5 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 204 |
| 5:40 PM | 0 | 6 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 202 |
| 5:45 PM | 1 | 2 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 190 |
| 5:50 PM | 0 | 4 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 187 |
| 5:55 PM | 0 | 8 | 0 | 0 | 2 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 19 | 192 |
| Peak 15-Min Flowrates | Northbound |  |  |  | Southbound |  |  |  | Eastbound |  |  |  | Westbound |  |  |  | Total |  |
|  | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U | Left | Thru | Right | U |  |  |  |
| All Vehicles | 0 | 92 | 0 | 0 | 0 | 176 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 0 |  | 76 |
| Heavy Trucks Buses | 0 | 16 | 0 |  | 0 | 40 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  |  | 6 |
| Pedestrians |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  | 0 |  |  |  | 0 |
| Bicycles Scooters | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  |  | 0 |

[^6]

Comments:


Comments:

## ATTACHMENT B - EXISTING TRAFFIC OPERATIONS WORKSHEETS

## Intersection Level Of Service Report Intersection 1: US 101 / Floras Creek Road

| Control Type: | Two-way stop | Delay (sec /veh): | 12.2 |
| :---: | :---: | :---: | :---: |
| Analysis Method: | HCM 7th Edition | Level Of Service: | B |
| Analysis Period: | 15 minutes | Volume to Capacity (v/c): | 0.004 |

Intersection Setup

| Name | US 101 |  | US 101 |  | Floras Creek Rd |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  | Southbound |  | Westbound |  |
| Lane Configuration |  |  | $4$ |  | $\stackrel{ }{ }$ |  |
| Turning Movement | Thru | Right | Left | Thru | Left | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 55.00 |  | 40.00 |  | 55.00 |  |
| Grade [\%] | 2.80 |  | -1.90 |  | 1.10 |  |
| Crosswalk | Yes |  | Yes |  | Yes |  |

## Volumes

| Name | US 101 |  | US 101 |  | Floras Creek Rd |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 195 | 4 | 7 | 263 | 2 | 9 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 11.00 | 0.00 | 0.00 | 8.00 | 0.00 | 11.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 195 | 4 | 7 | 263 | 2 | 9 |
| Peak Hour Factor | 0.9200 | 0.9200 | 0.9200 | 0.9200 | 0.9200 | 0.9200 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 53 | 1 | 2 | 71 | 1 | 2 |
| Total Analysis Volume [veh/h] | 212 | 4 | 8 | 286 | 2 | 10 |
| Pedestrian Volume [ped/h] | 0 |  | 0 |  | 0 |  |

Version 2022 (SP 0-2)
Intersection Settings

| Priority Scheme | Free | Free | Stop |
| :---: | :---: | :---: | :---: |
| Flared Lane |  |  | Yes |
| Storage Area [veh] | 0 | 0 | 0 |
| Two-Stage Gap Acceptance |  |  |  |
| Number of Storage Spaces in Median | 0 | 0 | No |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.00 | 0.00 | 0.01 | 0.00 | 0.00 | 0.01 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 0.00 | 0.00 | 7.64 | 0.00 | 12.24 | 9.59 |
| Movement LOS | A | A | A | A | B | A |
| 95th-Percentile Queue Length [veh/ln] | 0.00 | 0.00 | 0.01 | 0.01 | 0.05 | 0.05 |
| 95th-Percentile Queue Length [ft/ln] | 0.00 | 0.00 | 0.33 | 0.33 | 1.26 | 1.26 |
| d_A, Approach Delay [s/veh] | 0.00 |  | 0.21 |  | 10.03 |  |
| Approach LOS | A |  | A |  | B |  |
| d_I, Intersection Delay [s/veh] | 0.35 |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |

## Intersection Level Of Service Report Intersection 2: US 101 / Sixes River Road

| Control Type: | Two-way stop | Delay $(\mathrm{sec} / \mathrm{veh}):$ | 13.3 |
| :---: | :---: | :---: | :---: |
| Analysis Method: | HCM 7th Edition | Level Of Service: | B |
| Analysis Period: | 15 minutes | Volume to Capacity $(\mathrm{v} / \mathrm{c}):$ | 0.018 |

Intersection Setup

| Name | US 101 |  |  | US 101 |  |  | Private Driveway |  |  | Sixes River Rd |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  |  | Southbound |  |  | Eastbound |  |  | Westbound |  |  |
| Lane Configuration | $71$ |  |  | $71$ |  |  | $\uparrow$ |  |  | $\uparrow$ |  |  |
| Turning Movement | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 95.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 55.00 |  |  | 55.00 |  |  | 10.00 |  |  | 55.00 |  |  |
| Grade [\%] | 0.00 |  |  | 0.00 |  |  | 2.40 |  |  | 0.70 |  |  |
| Crosswalk | Yes |  |  | Yes |  |  | Yes |  |  | Yes |  |  |

## Volumes

| Name | US 101 |  |  | US 101 |  |  | Private Driveway |  |  | Sixes River Rd |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 2 | 215 | 10 | 6 | 221 | 2 | 0 | 0 | 2 | 7 | 0 | 10 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 0.00 | 12.00 | 10.00 | 0.00 | 13.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 20.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 2 | 215 | 10 | 6 | 221 | 2 | 0 | 0 | 2 | 7 | 0 | 10 |
| Peak Hour Factor | 0.8600 | 0.8600 | 0.8600 | 0.8600 | 0.8600 | 0.8600 | 0.8600 | 0.8600 | 0.8600 | 0.8600 | 0.8600 | 0.8600 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 1 | 63 | 3 | 2 | 64 | 1 | 0 | 0 | 1 | 2 | 0 | 3 |
| Total Analysis Volume [veh/h] | 2 | 250 | 12 | 7 | 257 | 2 | 0 | 0 | 2 | 8 | 0 | 12 |
| Pedestrian Volume [ped/h] |  | 0 |  |  | 0 |  |  | 1 |  |  | 0 |  |

Version 2022 (SP 0-2)
Intersection Settings

| Priority Scheme | Free | Free | Stop |  |
| :---: | :---: | :---: | :---: | :---: |
| Flared Lane |  |  | Stop |  |
| Storage Area [veh] | 0 | 0 | 0 |  |
| Two-Stage Gap Acceptance |  |  | 0 |  |
| Number of Storage Spaces in Median | 0 | 0 | No |  |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.00 | 0.00 | 0.00 | 0.01 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.02 | 0.00 | 0.02 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 7.74 | 0.00 | 0.00 | 7.75 | 0.00 | 0.00 | 13.69 | 13.64 | 9.69 | 13.27 | 13.36 | 10.10 |
| Movement LOS | A | A | A | A | A | A | B | B | A | B | B | B |
| 95th-Percentile Queue Length [veh/ln] | 0.00 | 0.00 | 0.00 | 0.02 | 0.00 | 0.00 | 0.01 | 0.01 | 0.01 | 0.11 | 0.11 | 0.11 |
| 95th-Percentile Queue Length [ft/ln] | 0.11 | 0.00 | 0.00 | 0.40 | 0.00 | 0.00 | 0.20 | 0.20 | 0.20 | 2.65 | 2.65 | 2.65 |
| d_A, Approach Delay [s/veh] |  | 0.06 |  |  | 0.20 |  |  | 9.69 |  |  | 11.37 |  |
| Approach LOS |  | A |  |  | A |  |  | A |  |  | B |  |
| d_I, Intersection Delay [s/veh] | 0.57 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |  |  |  |  |  |  |

## Intersection Level Of Service Report

Intersection 3: US 101 / Cape Blanco Highway (OR 250)

| Control Type: | Two-way stop | Delay (sec / veh): | 12.1 |
| :---: | :---: | :---: | :---: |
| Analysis Method: | HCM 7th Edition | Level Of Service: | B |
| Analysis Period: | 15 minutes | Volume to Capacity $(\mathrm{v} / \mathrm{c}):$ | 0.027 |

Intersection Setup

| Name | US 101 |  | US 101 |  | Cape Blanco Highway |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  | Southbound |  | Eastbound |  |
| Lane Configuration |  |  |  |  |  |  |
| Turning Movement | Left | Thru | Thru | Right | Left | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 1 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 180.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 55.00 |  | 55.00 |  | 55.00 |  |
| Grade [\%] | -1.30 |  | 2.50 |  | 1.50 |  |
| Crosswalk | Yes |  | Yes |  | Yes |  |

## Volumes

| Name |  |  |  |  | Cape | ghway |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 22 | 213 | 226 | 8 | 13 | 15 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 0.00 | 11.00 | 12.00 | 25.00 | 8.00 | 0.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 22 | 213 | 226 | 8 | 13 | 15 |
| Peak Hour Factor | 0.9300 | 0.9300 | 0.9300 | 0.9300 | 0.9300 | 0.9300 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 6 | 57 | 61 | 2 | 3 | 4 |
| Total Analysis Volume [veh/h] | 24 | 229 | 243 | 9 | 14 | 16 |
| Pedestrian Volume [ped/h] | 0 |  | 0 |  | 1 |  |

Version 2022 (SP 0-2)
Intersection Settings

| Priority Scheme | Free | Free | Stop |
| :---: | :---: | :---: | :---: |
| Flared Lane |  |  | No |
| Storage Area [veh] | 0 | 0 | 0 |
| Two-Stage Gap Acceptance |  |  |  |
| Number of Storage Spaces in Median | 0 | 0 | No |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.02 | 0.00 | 0.00 | 0.00 | 0.03 | 0.02 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 7.77 | 0.00 | 0.00 | 0.00 | 12.11 | 9.25 |
| Movement LOS | A | A | A | A | B | A |
| 95th-Percentile Queue Length [veh/ln] | 0.06 | 0.00 | 0.00 | 0.00 | 0.14 | 0.14 |
| 95th-Percentile Queue Length [ft/ln] | 1.39 | 0.00 | 0.00 | 0.00 | 3.48 | 3.48 |
| d_A, Approach Delay [s/veh] | 0.74 |  | 0.00 |  | 10.58 |  |
| Approach LOS | A |  | A |  | B |  |
| d_I, Intersection Delay [s/veh] | 0.94 |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |

## Intersection Level Of Service Report Intersection 4: US 101 / Ophir Creek Road

Control Type:
Analysis Method:
Analysis Period:
Two-way stop
HCM 7 th Edition
15 minutes

| Delay (sec / veh): | 11.3 |
| :---: | :---: |
| Level Of Service: | B |
| Volume to Capacity $(\mathrm{v} / \mathrm{c}):$ | 0.002 |

Intersection Setup

| Name | US 101 |  |  | US 101 |  |  | Private Driveway |  |  | Ophir Creek Road |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  |  | Southbound |  |  | Eastbound |  |  | Westbound |  |  |
| Lane Configuration | $\uparrow$ |  |  | $\uparrow$ |  |  | $\uparrow$ |  |  | $\uparrow$ |  |  |
| Turning Movement | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 55.00 |  |  | 55.00 |  |  | 15.00 |  |  | 55.00 |  |  |
| Grade [\%] | 0.72 |  |  | -1.50 |  |  | 1.80 |  |  | 1.50 |  |  |
| Crosswalk | Yes |  |  | Yes |  |  | Yes |  |  | Yes |  |  |

## Volumes

| Name | US 101 |  |  | US 101 |  |  | Private Driveway |  |  | Ophir Creek Road |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 0 | 104 | 1 | 4 | 161 | 0 | 0 | 0 | 0 | 0 | 1 | 3 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 0.00 | 17.00 | 0.00 | 0.00 | 16.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 0 | 104 | 1 | 4 | 161 | 0 | 0 | 0 | 0 | 0 | 1 | 3 |
| Peak Hour Factor | 0.8400 | 0.8400 | 0.8400 | 0.8400 | 0.8400 | 0.8400 | 0.8400 | 0.8400 | 0.8400 | 0.8400 | 0.8400 | 0.8400 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 0 | 31 | 0 | 1 | 48 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| Total Analysis Volume [veh/h] | 0 | 124 | 1 | 5 | 192 | 0 | 0 | 0 | 0 | 0 | 1 | 4 |
| Pedestrian Volume [ped/h] |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |

Version 2022 (SP 0-2)
Intersection Settings

| Priority Scheme | Free | Free | Stop |  |
| :---: | :---: | :---: | :---: | :---: |
| Flared Lane |  |  | No |  |
| Storage Area [veh] | 0 | 0 | 0 |  |
| Two-Stage Gap Acceptance |  |  | 0 |  |
| Number of Storage Spaces in Median | 0 | 0 | No |  |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 7.58 | 0.00 | 0.00 | 7.45 | 0.00 | 0.00 | 10.98 | 11.28 | 9.25 | 10.92 | 11.27 | 8.91 |
| Movement LOS | A | A | A | A | A | A | B | B | A | B | B | A |
| 95th-Percentile Queue Length [veh/ln] | 0.00 | 0.00 | 0.00 | 0.01 | 0.01 | 0.01 | 0.00 | 0.00 | 0.00 | 0.02 | 0.02 | 0.02 |
| 95th-Percentile Queue Length [ft/ln] | 0.00 | 0.00 | 0.00 | 0.21 | 0.21 | 0.21 | 0.00 | 0.00 | 0.00 | 0.46 | 0.46 | 0.46 |
| d_A, Approach Delay [s/veh] |  | 0.00 |  |  | 0.19 |  |  | 10.50 |  |  | 9.38 |  |
| Approach LOS |  | A |  |  | A |  |  | B |  |  | A |  |
| d_I, Intersection Delay [s/veh] | 0.26 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |  |  |  |  |  |  |

## Intersection Level Of Service Report

## Intersection 5: US 101 / Edson Creek Road-Nesika Road

Control Type: Analysis Method: Analysis Period:

Two-way stop
HCM 7th Edition
15 minutes

Delay (sec / veh):
Level Of Service:
Volume to Capacity ( $\mathrm{v} / \mathrm{c}$ ):
12.6

B
0.016

Intersection Setup

| Name | US 101 |  |  | US 101 |  |  | Nesika Road |  |  | Edson Creek Road |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  |  | Southbound |  |  | Eastbound |  |  | Westbound |  |  |
| Lane Configuration | $\uparrow$ |  |  | $\uparrow$ |  |  | $\uparrow$ |  |  | $\uparrow$ |  |  |
| Turning Movement | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 55.00 |  |  | 55.00 |  |  | 55.00 |  |  | 45.00 |  |  |
| Grade [\%] | 0.40 |  |  | 0.30 |  |  | 2.30 |  |  | -2.00 |  |  |
| Crosswalk | Yes |  |  | Yes |  |  | Yes |  |  | Yes |  |  |

## Volumes

| Name | US 101 |  |  | US 101 |  |  | Nesika Road |  |  | Edson Creek Road |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 20 | 119 | 6 | 8 | 159 | 1 | 0 | 7 | 16 | 5 | 4 | 4 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 10.00 | 18.00 | 0.00 | 25.00 | 16.00 | 0.00 | 0.00 | 14.00 | 6.00 | 0.00 | 0.00 | 50.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 20 | 119 | 6 | 8 | 159 | 1 | 0 | 7 | 16 | 5 | 4 | 4 |
| Peak Hour Factor | 0.8800 | 0.8800 | 0.8800 | 0.8800 | 0.8800 | 0.8800 | 0.8800 | 0.8800 | 0.8800 | 0.8800 | 0.8800 | 0.8800 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 6 | 34 | 2 | 2 | 45 | 0 | 0 | 2 | 5 | 1 | 1 | 1 |
| Total Analysis Volume [veh/h] | 23 | 135 | 7 | 9 | 181 | 1 | 0 | 8 | 18 | 6 | 5 | 5 |
| Pedestrian Volume [ped/h] |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |

Version 2022 (SP 0-2)
Intersection Settings

| Priority Scheme | Free | Free | Stop |  |
| :---: | :---: | :---: | :---: | :---: |
| Flared Lane |  |  | Stop |  |
| Storage Area [veh] | 0 | 0 | 0 |  |
| Two-Stage Gap Acceptance |  |  | 0 |  |
| Number of Storage Spaces in Median | 0 | 0 | No |  |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.02 | 0.00 | 0.00 | 0.01 | 0.00 | 0.00 | 0.00 | 0.02 | 0.02 | 0.01 | 0.01 | 0.01 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 7.70 | 0.00 | 0.00 | 7.76 | 0.00 | 0.00 | 12.08 | 12.55 | 9.48 | 11.64 | 11.57 | 9.64 |
| Movement LOS | A | A | A | A | A | A | B | B | A | B | B | A |
| 95th-Percentile Queue Length [veh/ln] | 0.04 | 0.04 | 0.04 | 0.02 | 0.02 | 0.02 | 0.12 | 0.12 | 0.12 | 0.08 | 0.08 | 0.08 |
| 95th-Percentile Queue Length [ft/ln] | 0.98 | 0.98 | 0.98 | 0.38 | 0.38 | 0.38 | 2.94 | 2.94 | 2.94 | 1.99 | 1.99 | 1.99 |
| d_A, Approach Delay [s/veh] |  | 1.07 |  |  | 0.37 |  |  | 10.43 |  |  | 10.99 |  |
| Approach LOS |  | A |  |  | A |  |  | B |  |  | B |  |
| d_I, Intersection Delay [s/veh] | 1.74 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |  |  |  |  |  |  |

## Intersection Level Of Service Report Intersection 6: US 101 / Pistol River Road

| Control Type: | Two-way stop | Delay (sec /veh): | 13.2 |
| :---: | :---: | :---: | :---: |
| Analysis Method: | HCM 7th Edition | Level Of Service: | B |
| Analysis Period: | 15 minutes | Volume to Capacity (v/c): | 0.013 |

Intersection Setup

| Name | US 101 |  | US 101 |  | Carpenterville Road |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  | Southbound |  | Westbound |  |
| Lane Configuration | $\stackrel{\rightharpoonup}{\Gamma}$ |  | $4$ |  | $\leftrightarrows$ |  |
| Turning Movement | Thru | Right | Left | Thru | Left | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 1 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 155.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 55.00 |  | 55.00 |  | 55.00 |  |
| Grade [\%] | -1.30 |  | 1.40 |  | 2.70 |  |
| Crosswalk | Yes |  | Yes |  | Yes |  |

## Volumes

| Name | US 101 |  | US 101 |  | Carpenterville Road |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 189 | 5 | 3 | 217 | 5 | 4 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 7.00 | 0.00 | 0.00 | 19.00 | 20.00 | 50.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 1 | 0 | 0 | 4 | 0 | 0 |
| Total Hourly Volume [veh/h] | 190 | 5 | 3 | 221 | 5 | 4 |
| Peak Hour Factor | 0.8100 | 0.8100 | 0.8100 | 0.8100 | 0.8100 | 0.8100 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 59 | 2 | 1 | 68 | 2 | 1 |
| Total Analysis Volume [veh/h] | 235 | 6 | 4 | 273 | 6 | 5 |
| Pedestrian Volume [ped/h] | 0 |  | 0 |  | 0 |  |

Version 2022 (SP 0-2)
Intersection Settings

| Priority Scheme | Free | Free | Stop |
| :---: | :---: | :---: | :---: |
| Flared Lane |  |  | Yes |
| Storage Area [veh] | 0 | 0 | 0 |
| Two-Stage Gap Acceptance |  |  |  |
| Number of Storage Spaces in Median | 0 | 0 | No |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.00 | 0.00 | 0.00 | 0.00 | 0.01 | 0.01 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 0.00 | 0.00 | 7.70 | 0.00 | 13.16 | 10.41 |
| Movement LOS | A | A | A | A | B | B |
| 95th-Percentile Queue Length [veh/ln] | 0.00 | 0.00 | 0.01 | 0.00 | 0.06 | 0.06 |
| 95th-Percentile Queue Length [ft/ln] | 0.00 | 0.00 | 0.22 | 0.00 | 1.58 | 1.58 |
| d_A, Approach Delay [s/veh] | 0.00 |  | 0.11 |  | 11.91 |  |
| Approach LOS | A |  | A |  | B |  |
| d_I, Intersection Delay [s/veh] | 0.31 |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |

## Intersection Level Of Service Report Intersection 7: US 101 / Cape Ferrelo Road

| Control Type: | Two-way stop | Delay (sec / veh): | 11.3 |
| :---: | :---: | :---: | :---: |
| Analysis Method: | HCM 7th Edition | Level Of Service: | B |
| Analysis Period: | 15 minutes | Volume to Capacity $(\mathrm{v} / \mathrm{c}):$ | 0.047 |

Intersection Setup

| Name | US 101 |  | US 101 |  | Cape Ferrelo Road |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  | Southbound |  | Westbound |  |
| Lane Configuration |  |  |  |  |  |  |
| Turning Movement | Thru | Right | Left | Thru | Left | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 55.00 |  | 55.00 |  | 35.00 |  |
| Grade [\%] | 0.00 |  | 0.00 |  | -7.10 |  |
| Crosswalk | Yes |  | Yes |  | Yes |  |

## Volumes

| Name | US 101 |  | US 101 |  | Cape Ferrelo Road |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 231 | 39 | 3 | 265 | 22 | 1 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 9.00 | 5.00 | 0.00 | 19.00 | 5.00 | 0.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 1 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 232 | 39 | 3 | 265 | 22 | 1 |
| Peak Hour Factor | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 | 0.7900 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 73 | 12 | 1 | 84 | 7 | 0 |
| Total Analysis Volume [veh/h] | 294 | 49 | 4 | 335 | 28 | 1 |
| Pedestrian Volume [ped/h] | 0 |  | 0 |  | 0 |  |

Version 2022 (SP 0-2)
Intersection Settings

| Priority Scheme | Free | Free | Stop |
| :---: | :---: | :---: | :---: |
| Flared Lane |  |  | No |
| Storage Area [veh] | 0 | 0 | 0 |
| Two-Stage Gap Acceptance |  |  |  |
| Number of Storage Spaces in Median | 0 | 0 | No |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.00 | 0.00 | 0.00 | 0.00 | 0.05 | 0.00 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 0.00 | 0.00 | 7.94 | 0.00 | 11.28 | 9.40 |
| Movement LOS | A | A | A | A | B | A |
| 95th-Percentile Queue Length [veh/ln] | 0.00 | 0.00 | 0.01 | 0.00 | 0.15 | 0.15 |
| 95th-Percentile Queue Length [ft/ln] | 0.00 | 0.00 | 0.17 | 0.08 | 3.75 | 3.75 |
| d_A, Approach Delay [s/veh] | 0.00 |  | 0.09 |  | 11.21 |  |
| Approach LOS | A |  | A |  | B |  |
| d_I, Intersection Delay [s/veh] | 0.50 |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |

## Intersection Level Of Service Report

Intersection 8: US 101 / Winchuck River Road-Ocean View Drive
Control Type:
Analysis Method:
Analysis Period:
Two-way stop
HCM 7th Edition
15 minutes

Delay (sec / veh):
32.1
Level Of Service:
Volume to Capacity (v/c):
D
0.096

Intersection Setup

| Name | US 101 |  |  | US 101 |  |  | Ocean View Dr |  |  | Winchuck River Rd |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach | Northbound |  |  | Southbound |  |  | Eastbound |  |  | Westbound |  |  |
| Lane Configuration | $7 F$ |  |  | $7 F$ |  |  | $\uparrow$ |  |  | $\uparrow$ |  |  |
| Turning Movement | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |
| Lane Width [ft] | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 | 12.00 |
| No. of Lanes in Entry Pocket | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Entry Pocket Length [ft] | 185.00 | 100.00 | 100.00 | 205.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |
| No. of Lanes in Exit Pocket | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Exit Pocket Length [ft] | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| Speed [mph] | 55.00 |  |  | 55.00 |  |  | 40.00 |  |  | 40.00 |  |  |
| Grade [\%] | 2.10 |  |  | -2.10 |  |  | 2.40 |  |  | -5.60 |  |  |
| Crosswalk | Yes |  |  | Yes |  |  | Yes |  |  | Yes |  |  |

## Volumes

| Name | US 101 |  |  | US 101 |  |  | Ocean View Dr |  |  | Winchuck River Rd |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Base Volume Input [veh/h] | 23 | 436 | 10 | 37 | 469 | 18 | 13 | 1 | 16 | 6 | 2 | 1 |
| Base Volume Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Heavy Vehicles Percentage [\%] | 4.00 | 2.00 | 10.00 | 6.00 | 7.00 | 0.00 | 0.00 | 0.00 | 0.00 | 20.00 | 0.00 | 8.00 |
| Growth Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| In-Process Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Site-Generated Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Diverted Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-by Trips [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Existing Site Adjustment Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Other Volume [veh/h] | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Hourly Volume [veh/h] | 23 | 436 | 10 | 37 | 469 | 18 | 13 | 1 | 16 | 6 | 2 | 1 |
| Peak Hour Factor | 0.9200 | 0.9200 | 0.9200 | 0.9200 | 0.9200 | 0.9200 | 0.9200 | 0.9200 | 0.9200 | 0.9200 | 0.9200 | 0.9200 |
| Other Adjustment Factor | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| Total 15-Minute Volume [veh/h] | 6 | 118 | 3 | 10 | 127 | 5 | 4 | 0 | 4 | 2 | 1 | 0 |
| Total Analysis Volume [veh/h] | 25 | 474 | 11 | 40 | 510 | 20 | 14 | 1 | 17 | 7 | 2 | 1 |
| Pedestrian Volume [ped/h] |  | 0 |  |  | 1 |  |  | 0 |  |  | 1 |  |

Version 2022 (SP 0-2)
Intersection Settings

| Priority Scheme | Free | Free | Stop |  |
| :---: | :---: | :---: | :---: | :---: |
| Flared Lane |  |  | Stop |  |
| Storage Area [veh] | 0 | 0 | 0 |  |
| Two-Stage Gap Acceptance |  |  | 0 |  |
| Number of Storage Spaces in Median | 0 | 0 | No |  |

Movement, Approach, \& Intersection Results

| V/C, Movement V/C Ratio | 0.02 | 0.00 | 0.00 | 0.04 | 0.01 | 0.00 | 0.10 | 0.01 | 0.03 | 0.03 | 0.01 | 0.00 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| d_M, Delay for Movement [s/veh] | 8.59 | 0.00 | 0.00 | 8.55 | 0.00 | 0.00 | 32.06 | 29.23 | 13.96 | 22.69 | 18.96 | 11.51 |
| Movement LOS | A | A | A | A | A | A | D | D | B | C | C | B |
| 95th-Percentile Queue Length [veh/ln] | 0.07 | 0.00 | 0.00 | 0.12 | 0.00 | 0.00 | 0.46 | 0.46 | 0.46 | 0.13 | 0.13 | 0.13 |
| 95th-Percentile Queue Length [ft/ln] | 1.87 | 0.00 | 0.00 | 2.95 | 0.00 | 0.00 | 11.38 | 11.38 | 11.38 | 3.28 | 3.28 | 3.28 |
| d_A, Approach Delay [s/veh] |  | 0.42 |  |  | 0.60 |  |  | 22.36 |  |  | 20.82 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | C |  |
| d_I, Intersection Delay [s/veh] | 1.32 |  |  |  |  |  |  |  |  |  |  |  |
| Intersection LOS | D |  |  |  |  |  |  |  |  |  |  |  |

## ATTACHMENT C - CRASH DATA AND INTERSECTION CRASH ANALYSIS WORKSHEETS

Intersectional Crashes at US-101, Oregon Coast Hwy (\#009) \& Cape Blanco Hwy (\#250) in Curry County, OR.
January 1, 2015 through December 31, 2020


YEAR.
TOTAL
FINAL TOTAL

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers,
see https://www.oregon.gov/ODOT/Data/documents/Crash_Data_Disclaimers.pdf.

Intersectional Crashes at US-101, Oregon Coast Hwy (\#009) \& Cape Ferrelo Rd in Curry County, OR.
January 1, 2015 through December 31, 2020

| COLLISION TYPE | FATAL CRASHES | $\begin{array}{r} \text { NON- } \\ \text { FATAL } \\ \text { CRASHES } \end{array}$ | PROPERTY DAMAGE ONLY | TOTAL CRASHES | $\begin{aligned} & \text { PEOPLE } \\ & \text { KILLED } \end{aligned}$ | PEOPLE <br> INJURED | TRUCKS | $\begin{aligned} & \text { DRY } \\ & \text { SURF } \end{aligned}$ | $\begin{aligned} & \text { WET } \\ & \text { SURF } \end{aligned}$ | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

YEAR.
TOTAL
FINAL TOTAL

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers,
see https://www.oregon.gov/ODOT/Data/documents/Crash Data Disclaimers.pdf.

## CRASH SUMMARIES BY YEAR BY COLLISION TYPE

Intersectional Crashes at US-101, Oregon Coast Hwy (\#009) \& Edson Creek Rd / Nesika Rd in Curry County, OR.
January 1, 2015 through December 31, 2020

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE <br> INJURED | TRUCKS | $\begin{gathered} \text { DRY } \\ \text { SURF } \end{gathered}$ | $\begin{aligned} & \text { WET } \\ & \text { SURF } \end{aligned}$ | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2020 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ANGLE | 0 | 0 | 1 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| REAR-END | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2020 TOTAL | 0 | 0 | 2 | 2 | 0 | 0 | 1 | 2 | 0 | 2 | 0 | 2 | 0 | 0 |
| YEAR: 2017 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| TURNING MOVEMENTS | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2017 TOTAL | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| FINAL TOTAL | 0 | 0 | 3 | 3 | 0 | 0 | 1 | 3 | 0 | 3 | 0 | 3 | 0 | 0 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers, see https://www.oregon.gov/ODOT/Data/documents/Crash_Data_Disclaimers.pdf.

## CRASH SUMMARIES BY YEAR BY COLLISION TYPE

Intersectional Crashes at US-101, Oregon Coast Hwy (\#009) \& Floras Creek Rd in Curry County, OR.
January 1, 2015 through December 31, 2020

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE KILLED | PEOPLE INJURED | TRUCKS | $\begin{aligned} & \text { DRY } \\ & \text { SURF } \end{aligned}$ | WET <br> SURF | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2017 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 1 | 0 | 1 | 0 | 3 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 0 |
| TURNING MOVEMENTS | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2017 TOTAL | 0 | 2 | 0 | 2 | 0 | 4 | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 0 |
| FINAL TOTAL | 0 | 2 | 0 | 2 | 0 | 4 | 0 | 1 | 1 | 2 | 0 | 2 | 0 | 0 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers,
see https://www.oregon.gov/ODOT/Data/documents/Crash_Data_Disclaimers.pdf.

Intersectional Crashes at US-101, Oregon Coast Hwy (\#009) \& Ophir Rd in Curry County, OR.
January 1, 2015 through December 31, 2020

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE <br> INJURED | TRUCKS | $\begin{gathered} \text { DRY } \\ \text { SURF } \end{gathered}$ | $\begin{aligned} & \text { WET } \\ & \text { SURF } \end{aligned}$ | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2018 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| REAR-END | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2018 TOTAL | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| FINAL TOTAL | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers
see https://www.oregon.gov/ODOT/Data/documents/Crash_Data_Disclaimers.pdf.

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

## CRASH SUMMARIES BY YEAR BY COLLISION TYPE

Intersectional Crashes at US-101, Oregon Coast Hwy (\#009) \& Sixes River Rd in Curry County, OR January 1, 2015 through December 31, 2020

| COLLISION TYPE | FATAL CRASHES | NON- <br> FATAL <br> CRASHES | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE <br> INJURED | TRUCKS | $\begin{gathered} \text { DRY } \\ \text { SURF } \end{gathered}$ | $\begin{aligned} & \text { WET } \\ & \text { SURF } \end{aligned}$ | DAY | DARK | INTERSECTION | INTERSECTION RELATED | OFF- <br> ROAD |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2020 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| TURNING MOVEMENTS | 0 | 1 | 0 | 1 | 0 | 1 | 1 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2020 TOTAL | 0 | 1 | 0 | 1 | 0 | 1 | 1 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| FINAL TOTAL | 0 | 1 | 0 | 1 | 0 | 1 | 1 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers
see https://www.oregon.gov/ODOT/Data/documents/Crash_Data_Disclaimers.pdf.

## CRASH SUMMARIES BY YEAR BY COLLISION TYPE

Intersectional Crashes at US-101, Oregon Coast Hwy (\#009) \& Winchuck River Rd / Ocean View Dr in Curry County, OR.
January 1, 2015 through December 31, 2020

| COLLISION TYPE | FATAL CRASHES | $\begin{gathered} \text { NON- } \\ \text { FATAL } \\ \text { CRASHES } \end{gathered}$ | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE <br> INJURED | TRUCKS | $\begin{gathered} \text { DRY } \\ \text { SURF } \end{gathered}$ | WET SURF | DAY | DARK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| YEAR: 2020 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ANGLE | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| 2020 TOTAL | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |
| FINAL TOTAL | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 |

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers,
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## CRASH SUMMARIES BY YEAR BY COLLISION TYPE

Intersectional Crashes at US-101, Oregon Coast Hwy (\#009) - MP 339.71 \& Carpenterville Hwy (\#255) in Curry County, OR.
January 1, 2015 through December 31, 2020

|  | FATAL CRASHES | $\begin{gathered} \text { NON- } \\ \text { FATAL } \\ \text { CRASHES } \end{gathered}$ | PROPERTY DAMAGE ONLY | TOTAL CRASHES | PEOPLE <br> KILLED | PEOPLE <br> INJURED |  | $\begin{aligned} & \text { DRY } \\ & \text { SURF } \end{aligned}$ | $\begin{aligned} & \text { WET } \\ & \text { SURF } \end{aligned}$ | DAY | RK | INTERSECTION | INTERSECTION RELATED | $\begin{aligned} & \text { OFF- } \\ & \text { ROAD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

YEAR.

TOTAL
FINAL TOTAL

Disclaimers: Effective 2016, collection of "Property Damage Only" (PDO) crash data elements was reduced for vehicles and participants. Age, Gender, License, Error and other elements are no longer available for PDO crash reporting. Please keep this in mind when comparing 2016 PDO crash data to prior years.

A higher number of crashes may be reported as of 2011 compared to prior years. This does not necessarily reflect an increase in annual crashes. The higher numbers may result from a change to an internal departmental process that allows the Crash Analysis and Reporting Unit to add previously unavailable, non-fatal crash reports to the annual data file. Please be aware of this change when comparing pre-2011 crash statistics. For all disclaimers,
see https://www.oregon.gov/ODOT/Data/documents/Crash_Data_Disclaimers.pdf.

|  | Location | Collision Type |  |  |  |  | Severity |  | Total Crashes | 90th Percentile Crash Rate | Observed <br> Crash Rate | Does Observed Exceed 90th Rate? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ID |  | Rear-end | Turning | Angle | Fixed | SS (M) | PDO | Injury |  |  |  |  |
| 1 | US 101/ Floras Creek Road | 1 | 1 |  |  |  |  | 2 | 2 | 0.48 | 0.23 | No |
| 2 | US 101/ Sixes River Road |  | 1 |  |  |  |  | 1 | 1 | 1.08 | 0.12 | No |
| 3 | US 101/ Cape Blanco Highway |  |  |  |  |  |  |  | 0 | 0.48 | 0.00 | No |
| 4 | US 101/ Ophir Road | 1 |  |  |  |  |  | 1 | 1 | 0.48 | 0.20 | No |
| 5 | US 101/ Edson Creek Road-Nesika Road | 1 | 1 | 1 |  |  | 3 |  | 3 | 1.08 | 0.47 | No |
| 6 | US 101/ Pistol River Road |  |  |  |  |  |  |  | 0 | 0.48 | 0.00 | No |
| 7 | US 101/ Cape Ferrelo Road |  |  |  |  |  |  |  | 0 | 0.48 | 0.00 | No |
| 8 | US 101/ Winchuck River Road |  |  | 1 |  |  |  | 1 | 1 | 1.08 | 0.05 | No |


|  | Location | PM Peak |  |  |  |  |  |  | Intersection Class | 90th Percentile Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ID |  | Day one | Day Two | Day Three | AVG | EST AADT | EST 5Y TEV | Crash Rate |  |  |
| 1 | US 101/ Floras Creek Road |  |  |  | 480 | 4800 | 8760000 | 0.23 | Rural 3ST | 0.475 |
| 2 | US 101/ Sixes River Road |  |  |  | 476 | 4760 | 8687000 | 0.12 | Rural 4ST | 1.08 |
| 3 | US 101/ Cape Blanco Highway |  |  |  | 497 | 4970 | 9070250 | 0.00 | Rural 3ST | 0.475 |
| 4 | US 101/ Ophir Road |  |  |  | 273 | 2730 | 4982250 | 0.20 | Rural 3ST | 0.475 |
| 5 | US 101/ Edson Creek Road-Nesika Road |  |  |  | 349 | 3490 | 6369250 | 0.47 | Rural 4ST | 1.08 |
| 6 | US 101/ Pistol River Road |  |  |  | 423 | 4230 | 7719750 | 0.00 | Rural 3ST | 0.475 |
| 7 | US 101/ Cape Ferrelo Road |  |  |  | 561 | 5610 | 10238250 | 0.00 | Rural 3ST | 0.475 |
| 8 | US 101/ Winchuck River Road |  |  |  | 1066 | 10660 | 19454500 | 0.05 | Rural 4ST | 1.08 |

PM Peak hour TEV from network tool

$$
\text { Intersection Crash Rate per } M E V=\frac{\text { Annual Number of Crashes } x 10^{6}}{(A A D T) x(365 \text { days } / \text { year })}
$$

The values shown in Exhibit 4-1 represent the $90^{\text {th }}$ percentile crash rates from a study of 500 intersections in Oregon. The crash rates are grouped by rural/urban, signalized/unsignalized, and three-leg/four-leg intersections. Intersections with crash rates that exceed the $90^{\text {th }}$ percentile values shown in the table should be flagged for further analysis. For more information on crash rates and using this table, see Section 4.3.4 Critical Crash Rate.

Exhibit 4-1: Intersection Crash Rates per MEV by Land Type and Traffic Control

|  | Rural |  |  |  | Urban |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3SG | 3ST | 4SG | 4ST | 3SG | 3ST | 4SG | 4ST |
| No. of Intersections | 7 | 115 | 20 | 60 | 55 | 77 | 106 | 60 |
| Mean Crash Rate | 0.226 | 0.196 | 0.324 | 0.434 | 0.275 | 0.131 | 0.477 | 0.198 |
| Median Crash Rate | 0.163 | 0.092 | 0.320 | 0.267 | 0.252 | 0.105 | 0.420 | 0.145 |
| Standard Deviation | 0.185 | 0.314 | 0.223 | 0.534 | 0.155 | 0.121 | 0.273 | 0.176 |
| Coefficient of Variation | 0.819 | 1.602 | 0.688 | 1.230 | 0.564 | 0.924 | 0.572 | 0.889 |
| $\mathbf{9 0}^{\text {th }}$ Percentile Rate | 0.464 | 0.475 | 0.579 | 1.080 | 0.509 | 0.293 | 0.860 | 0.408 |

Source: Assessment of Statewide Intersection Safety Performance, FHWA-OR-RD-18, Portland State
University and Oregon State University, June 2011, Table 4.1, p. 47.
Note: Traffic control types include
3SG (three-leg signalized),
3ST (three-leg minor stop-control),
4SG (four-leg signalized),
4ST (four-leg minor stop-control).

| General \& Site Information |  |
| :--- | :--- |
| Analyst: | Sophia Semensky |
| Agency/Company: | Kittelson \& Associates |
| Date: | $1 / 19 / 2023$ |
| Project Name: | Curry County TSP |



| Average Crash Rate per intersection type |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection Pop. Type | Sum of <br> Crashes | Sum of 5- <br> year MEV | Avg Crash <br> Rate for Ref <br> Pop. | INT in Pop |  |
| Rural 3SG | 0 | 0 |  |  |  |
| Rural 3ST | 3 | 41 | 0.0736 | 5 |  |
| Rural 4SG | 0 | 0 |  |  |  |
| Rural 4ST | 5 | 35 | 0.1449 | 3 |  |
| Urban 3ST | 0 | 0 |  |  |  |
| Urban 3SG | 0 | 0 |  |  |  |
| Urban 4ST | 0 | 0 |  |  |  |
| Urban 4SG | 0 | 0 |  |  |  |


| Critical Rate Calculation |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection | AADT Entering Intersection | 5-year MEV | Crash Total | Intersection Population Type | Intersection Crash Rate | Reference Population Crash Rate | Critical Rate | Over Critical |
| US 101/ Floras Creek Road | 4,800 | 8.8 | 2 | Rural 3ST | 0.23 | 0.07 | 0.28 | Under |
| US 101/ Sixes River Road | 4,760 | 8.7 | 1 | Rural 4ST | 0.12 | APM Exhibit 4-1 |  |  |
| US 101/ Cape Blanco Highway | 4,970 | 9.1 | 0 | Rural 3ST | 0.00 | 0.07 | 0.28 | Under |
| US 101/ Ophir Road | 2,730 | 5.0 | 1 | Rural 3ST | 0.20 | 0.07 | 0.37 | Under |
| US 101/ Edson Creek Road-Nesika Road | 3,500 | 6.4 | 3 | Rural 4ST | 0.47 | APM Exhibit 4-1 |  |  |
| US 101/ Pistol River Road | 4,230 | 7.7 | 0 | Rural 3ST | 0.00 | 0.07 | 0.30 | Under |
| US 101/ Cape Ferrelo Road | 5,610 | 10.2 | 0 | Rural 3ST | 0.00 | 0.07 | 0.26 | Under |
| US 101/ Winchuck River Road | 10,650 | 19.4 | 1 | Rural 4ST | 0.05 | APM Exhibit 4-1 |  |  |
|  |  |  |  |  |  |  |  |  |



|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hwy | MP | Reference Pop | Street 1 | Street 2 | Angle | Back | Bike | Fix | Head | NonCol | $\frac{\text { Type of Cra }}{\text { OTH }}$ | Park | Ped | SS-M | ss-o | Turn | Rear | 32 |
| US 101/ Floras Creek Road | 0.00 | Rural 3ST | US 101 | Floras Creek Road |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| US 101/ Sixes River Road | 0.00 | Rural 4ST | US 101 | Sixes River Road |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| US 101/ Cape Blanco Highway | 0.00 | Rura 3ST | US 101 | Cape Blanco Highwa |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $\frac{\text { US } 101 / \text { Ophir Road }}{\text { S } 101 \text { Edson Creek RoadNesika }}$ | 0.00 0.00 | $\frac{\text { Rural } 4 \text { ST }}{\text { Rural } 4 \text { ST }}$ | US 101 | Ophir Road |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| US 101/ Pistol River Read | 0.00 | Rural 3ST | US 101 | Pistol River Road |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| US 101/ Cape Ferrelo Road | 0.00 | Rural 3ST | US 101 | Cape Ferrelo Road |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| US 101/ Winchuck River Road | 0.00 | Rural 4ST | US 101 | Winchuck River Road |  |  |  |  |  |  |  |  |  |  |  |  |  |  |



## ATTACHMENT C - SEGMENT CRASH ANALYSIS WORKSHEETS

|  | Street | From | то | Distance (mi) | ADT | Area | Functional Classification | Intersection Crashes | $\begin{gathered} \text { Total } \\ \text { Crashes } \end{gathered}$ | Rural Highway System Crash Rate | Est. 5-Year AADT | Observed Crash Rate | Does Observed Exceed 90th Rate? |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Winchuk River Road | US 101 | MP 7.0 | 4.1 | 590 | Rural Area | Rural Major Collector | 0 | 4 | 1.44 | 4408215 | 0.91 | No |
| 2 | Winchuk River Road | MP 7.0 | Wheeler Creek Rd | 3.3 | 200 | Rural Area | Rural Major Collector | 0 | 1 | 1.44 | 1205230 | 0.83 | No |
| 3 | Oceanview Dr | US 101 | Seagull Ln | 1.4 | 960 | Rural Area | Rural Minor Collector |  | 2 | 1.85 | 2487840 | 0.80 | No |
| 4 | Oceanview Dr | Seagull Ln | Max Ln | 0.2 | 850 | Rural Area | Rural Minor Collector | 0 | 0 | 1.85 | 266815 | 0.00 | No |
| 5 | Oceanview Dr | Max Ln | Wenbourne Ln | 1.7 | 1270 | Rural Cities | Rural Minor Collector | 1 | 6 | 3.74 | 3940175 | 1.52 | No |
| 6 | Oceanview Dr | Wenbourne Ln | W Benham Ln | 0.2 | 260 | Rural Cities | Rural Minor Collector | 1 (double counted with benham) | 1 | 3.74 | 109135 | 9.16 | Yes |
| 7 | Benham Ln | Oceanview Dr | US 101 | 0.5 | 3510 | Rural Cities | Rural Major Collector | 1 (double counted with oceanview) | 2 | 1.45 | 3459105 | 0.58 | No |
| 8 | Benham Ln | Oceanview Dr | Lower Harbor Rd | 0.2 | 2380 | Rural Cities | Rural Major Collector | 2 (double counted with benham/oceanview and with benham/lower | 4 | 1.45 | 651525 | 6.14 | Yes |
| 9 | S Bank Chetco River Rd | US 101 | Harbor View Creek | 0.2 | 2650 | Rural Area | Rural Major Collector | 2 | 2 | 1.44 | 1063975 | 1.88 | Yes |
| 10 | S Bank Chetco River Rd | Harbor View Creek | Campbell Rd | 3.0 | 1160 | Rural Area | Rural Major Collector | 0 | 3 | 1.44 | 6266320 | 0.48 | No |
| 11 | S Bank Chetco River Rd | Campbell Rd | UGB (MP 4.0) | 0.9 | 560 | Rural Area | Rural Major Collector | 0 | 1 | 1.44 | 930020 | 1.08 | No |
| 12 | S Bank Chetco River Rd | UGB (MP 4.0) | Eastern Terminus | 2.1 | 90 | Rural Area | Rural Major Collector | 0 | 1 | 1.44 | 344925 | 2.90 | Yes |
| 13 | Lower Harbor Rd | Benham Ln | Shopping Center Ave | 0.8 | 4580 | Rural Cities | Rural Major Collector | ${ }^{\text {a }}$, | 6 | 1.45 | 6603215 | 0.91 | No |
| 14 | Lower Harbor Rd | Shopping Center Ave | US 101 | 0.2 | 5910 | Rural Cities | Rural Major Collector | 1 | 1 | 1.45 | 1833578 | 0.55 | No |
| 15 | Shopping Center Ave | W Hoffeldt Ln | Lower Harbor Rd | 0.6 | 1940 | Rural Cities | Rural Major Collector | 6 (1 double counted with lower harbor) | 9 | 1.45 | 2159705 | 4.17 | Yes |
| 16 | N Bank Chetco River Rd | MP 1 | Yellowbrick Rd | 2.2 | 2500 | Rural Area | Rural Major Collector | 1 | 8 | 1.44 | 10220000 | 0.78 | No |
| 17 | N Bank Chetco River Rd | Yellowbrick Rd | UGB (MP 5) | 1.6 | 1270 | Rural Area | Rural Major Collector | 1 | 7 | 1.44 | 3638868 | 1.92 | Yes |
| 18 | N Bank Chetco River Rd | UGB (MP 5) | Gardner Ridge Rd | 0.7 | 1030 | Rural Area | Rural Major Collector | 6 | 6 | 1.44 | 1221838 | 4.91 | Yes |
| 19 | N Bank Chetco River Rd | Gardner Ridge Rd | MP 8.5 | 2.9 | 300 | Rural Area | Rural Minor Collector | 0 | 5 | 1.85 | 1604175 | 3.12 | Yes |
| 20 | Gardner Ridge Rd | N Bank Chetco River Rd | Palmer Butte | 3.4 | 260 | Rural Area | Rural Minor Collector | 1 | 3 | 1.85 | 1627535 | 1.84 | No |
| 21 | Gardner Ridge Rd | Palmer Butte | Hazel Camp Road | 3.5 | 80 | Rural Area | Rural Minor Collector | 0 | 3 | 1.85 | 516840 | 5.80 | Yes |
| 22 | Gardner Ridge Rd | Hazel Camp Road | Summer Bridge Rd | 4.6 | 60 | Rural Area | Rural Minor Collector | 0 | 0 | 1.85 | 501510 | 0.00 | No |
| 23 | Cape Ferrelo Rd | US 101 | Brookside Dr | 1.3 | 980 | Rural Area | Rural Major Collector | 0 | 1 | 1.44 | 2235625 | 0.45 | No |
| 24 | Cape Ferrelo Rd | Brookside Dr | HWY 255 | 1.3 | 419 | Rural Area | Rural Major Collector | 0 | 1 | 1.44 | 1023849 | 0.98 | No |
| 25 | Pistol River Loop | HWY 255 | Hwy 255 (Cape View Loop) | 1.9 | 167 | Rural Area | Rural Major Collector | 0 | 1 | 1.44 | 566882 | 1.76 | Yes |
| 26 | N Bank Pistol River Rd | Pistol River Loop | MP 3.8 | 3.8 | 62 | Rural Area | Rural Minor Collector | 0 | 0 | 1.85 | 428747 | 0.00 | No |
| 27 | N Bank Pistol River Rd | MP 3.8 | MP 8 (Forest Boundary) | 3.9 | 50 | Rural Area | Rural Minor Collector | 0 | 0 | 1.85 | 359525 | 0.00 | No |
| 28 | Hunter Creek Rd | HWY 009 | UGB (MP 2.3) | 1.5 | 3565 | Rural Cities | Rural Major Collector | 2 | 4 | 1.45 | 9889310 | 0.40 | No |
| 29 | Hunter Creek Rd | UGB (MP 2.3) | Eastern Terminus | 3.4 | 217 | Rural Area | Rural Major Collector | 0 | 1 | 1.44 | 1346485 | 0.74 | No |
| 30 |  | Hunter Creek Rd | Agness Rd | 2.6 | 50 | Rural Area | Rural Major Collector | 0 | 0 | 1.44 | 239075 | 0.00 | No |
| 31 | Jerrys Flat Rd | US 101 | Saunders Creek Rd | 2.9 | 1809 | Rural Area | Minor Arterial | 0 | 6 | 1.22 | 9408021 | 0.64 | No |
| 32 | Jerrys Flat Rd | Saunders Creek Rd | UGB (MP 75.5) | 1.7 | 586 | Rural Area | Minor Arterial | 0 | 2 | 1.22 | 1850149 | 1.08 | No |
| 33 | Jerrys Flat Rd | UGB (MP 75.5) | Eastern Terminus | 4.9 | 289 | Rural Area | Minor Arterial | 0 | 6 | 1.22 | 2600972 | 2.31 | Yes |
| 34 | Agness Rd | Couger Ln | Agness Rd Fork | 3.3 | 71 | Rural Area | Rural Minor Collector | 0 | 1 | 1.85 | 424586 | 2.36 | Yes |
| 35 | Agness Rd | Agness Rd Fork | $N$ of Billings Rd | 4.8 | 51 | Rural Area | Rural Minor Collector | 0 | 3 | 1.85 | 438694 | 6.84 | Yes |
| 36 | Oak Flat Rd | Agness Rd | campground road (MP 3) | 3.2 | 54 | Rural Area | Rural Minor Collector | 0 | 2 | 1.85 | 313389 | 6.38 | Yes |
| 37 | N Bank Rogue River Rd | US 101 | Edson Creek Rd | 3.7 | 1453 | Rural Area | Rural Major Collector | 0 | 7 | 1.44 | 9890934 | 0.71 | No |
| 38 | N Bank Rogue River Rd | Edson Creek Rd | Cedar Valley Rd | 1.3 | 1121 | Rural Area | Rural Major Collector | 0 | 3 | 1.44 | 2658386 | 1.13 | No |
| 39 | N Bank Rogue River Rd | Cedar Valley Rd | Bluebird Ln | 3.2 | 428 | Rural Area | Rural Major Collector | 0 | 4 | 1.44 | 2500980 | 1.60 | Yes |
| 40 | N Bank Rogue River Rd | Bluebird Ln | Lobster Creek Rd | 2.6 | 166 | Rural Area | Rural Major Collector | 0 | 1 | 1.44 | 773216 | 1.29 | No |
| 41 | Cedar Valley Rd | N Bank Rogue River Rd | Sidney Way | 3.9 | 193 | Rural Area | Rural Major Collector | 0 | 4 | 1.44 | 1356066 | 2.95 | Yes |
| 42 | Cedar Valley Rd | Sidney Way | Ophir Rd | 4.0 | 176 | Rural Area | Rural Major Collector | 0 | 1 | 1.44 | 1282367 | 0.78 | No |
| 43 | Edson Creek Rd | N Bank Rogue River Rd | US 101 | 2.3 | 308 | Rural Area | Rural Major Collector | $\begin{aligned} & 1 \text { (double count } w \text { US } \\ & \text { 101) } \end{aligned}$ | 2 | 1.44 | 1319551 | 1.52 | Yes |
| 44 | Nesika Beach Rd | US 101 | Edson Creek Rd | 1.2 | 74 | Rural Area | Rural Minor Collector | 2 (double count w Edson, US 101/Ophir) | 3 | 1.85 | 166708 | 18.00 | Yes |
| 45 | Ophir Rd | US 101 | US 101 | 4.3 | 164 | Rural Area | Rural Minor Collector | ${ }^{\text {TuOuve }}$ (0untrowos | 5 | 1.85 | 1275018 | 3.92 | Yes |
| 46 | Euchre Creek Rd | Ophir Rd | MP 3 (Forest Road) | 1.9 | 52 | Rural Area | Rural Major Collector | 0 | 0 | 1.44 | 180310 | 0.00 | No |
| 48 | Elk River Rd | US 101 | Wagner Ln | 2.1 | 464 | Rural Area | Rural Major Collector | 0 |  | 1.44 | 1779558 | 1.69 | Yes |
| 49 | Elk River Rd | Wagner Ln | Haiku Ln (MP 5.5) | 3.6 | 204 | Rural Area | Rural Major Collector | 0 | 2 | 1.44 | 1332834 | 1.50 | Yes |
| 50 | Elk River Rd/NF 5325 | Haiku Ln (MP 5.5) | County Boundary (Coos) | 1.9 | 125 | Rural Area | Rural Major Collector | 0 | 1 | 1.44 | 428875 | 2.33 | Yes |
| 51 | Sixes River Rd | US 101 | Edson Creek Park Rd | 4.3 | 247 | Rural Area | Rural Major Collector | - | 1 | 1.44 | 1920302 | 0.52 | No |
| 52 | Sixes River Rd | Edson Creek Park Rd | Plum Tree Rd | 4.3 | 74 | Rural Area | Rural Major Collector | 0 | 0 | 1.44 | 578014 | 0.00 | No |
| 53 | Sixes River Rd | Plum Tree Rd | USFS 10 Rd | 2.0 | 21 | Rural Area | Rural Minor Collector | ${ }_{\text {ctuouvt }}^{0}$ | 0 | 1.85 | 76650 | 0.00 | No |
| 54 | Airport Rd | US 101 | The Airport | 2.9 | 90 | Rural Area | Rural Major Collector |  |  | 1.44 | 474683 | 4.21 | Yes |
| 55 | Floras Lake Loop Rd | US 1015 | Us 101 N | 4.1 | 196 | Rural Area | Rural Major Collector |  | 5 | 1.44 | 1467647 | 3.41 | Yes |
| 56 | Floras Lake Rd | Floras Lake Loop Rd | Lakes End Dr | 1.9 | 181 | Rural Area | Rural Minor Collector | 0 | 0 | 1.85 | 634224 | 0.00 | No |
| 57 | Floras Creek Rd | US 101 | Clear Creek | 3.9 | 267 | Rural Area | Rural Major Collector | 0 | 0 | 1.44 | 1905245 | 0.00 | No |
| 58 | Floras Creek Rd | Clear Creek | S Fork Flores Creek Rd | 4.7 | 136 | Rural Area | Rural Major Collector | 0 | 0 | 1.44 | 1171504 | 0.00 | No |
| 59 | Langlois Mountain Rd | US 101 | MP 1.7 | 4.2 | 114 | Rural Area | Rural Minor Collector | 1Tuouvir counteuwtrn | 0 | 1.85 | 871255 | 0.00 | No |
| 60 | Langlois Mountain Rd | MP 1.7 | Mcleod Rd | 3.6 | 100 | Rural Area | Rural Minor Collector | 0 | 1 | 1.85 | 651525 | 1.53 | No |


| 61 | Langlois Mountain Rd | Mcleod Rd | Bennet Butte Rd | 1.8 | 32 | Rural Area | Rural Minor Collector | 0 | 0 | 1.85 | 103368 | 0.00 | No |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 62 | Pedrioli Dr | US 101 | Western Terminus | 0.9 | 539 | Rural Cities | Rural Minor Collector | ग<uOuvecturnteuwtr | 5 | 3.74 | 874659 | 5.72 | Yes |
| 63 | W Hoffeldt Ln | South of Titus Ln | US 101 | 0.4 | 1064 | Rural Cities | Rural Minor Collector | 7 (6 with US 101) | 9 | 3.74 | 718128 | 12.53 | Yes |
| 64 | Old County Rd | Pacific Terrace Loop | Eastern Terminus | 2.1 | 188 | Rural Area | Rural Minor Collector | 0 | 1 | 1.85 | 703355 | 1.42 | No |
| 65 | Parkview Dr | Vistra Ridge Dr | Eastern Terminus | 0.7 | 68 | Rural Cities | Rural Minor Collector | 0 | 0 | 3.74 | 81906 | 0.00 | No |
| 66 | Rainbow Rock Rd | Carpenterville Hwy | Aqua Vista Ln | 1.0 | 801 | Rural Cities | Rural Minor Collector | 0 | 3 | 3.74 | 1461825 | 2.05 | No |
| 67 | Rainbow Rock Rd | Aqua Vista Ln | Carpenterville Hwy | 1.1 | 121 | Rural Area | Rural Minor Collector | 0 | 1 | 1.85 | 249532 | 4.01 | Yes |
| 69 | Wedderburn Loop | US 101 | Old Coast Hwy | 1.3 | 337 | Rural Area | Rural Minor Collector | 0 | 1 | 1.85 | 787232 | 1.27 | No |
| 70 | Old Coast Hwy | Wedderburn Loop | US 101 | 2.0 | 40 | Rural Area | Rural Minor Collector |  | 2 | 1.85 | 148190 | 13.50 | Yes |
| 71 | China Mountain Rd | US 101 | NF-020 | 1.8 | 228 | Rural Area | Rural Minor Collector | 0 | 1 | 1.85 | 765624 | 1.31 | No |
| 72 | China Mountain Rd | NF-020 | UGB | 3.8 | 228 | Rural Area | Rural Minor Collector | 0 | 1 | 1.85 | 1581180 | 0.63 | No |
| 73 | Cemetery Loop Rd | US 101 | US 101 | 1.3 | 253 | Rural Cities | Rural Minor Collector | 0 | 0 | 3.74 | 618712 | 0.00 | No |
| 74 | Vista Dr | Gold Run Rd | Old Mill Rd | 0.7 | 86 | Rural Cities | Rural Minor Collector | 0 | 0 | 3.74 | 101622 | 0.00 | No |
| 75 | Grassy Knob Rd | US 101 | Eastern Terminus | 4.2 | 180 | Rural Area | Rural Minor Collector | 0 | 0 | 1.85 | 1382985 | 0.00 | No |
| 76 | US 101 | CA border | Oceanview Dr | 0.9 | 8803 | Rural Area | Other Principal Arterials | 3 (1 with Oceandrive) | 11 | 1.22 | 14137618 | 0.78 | No |
|  | US 101 | Oceanview Dr | McKay Creek Rd | 1.0 | 10318 | Rural Area | Other Principal Arterials | 9 | 12 | 1.22 | 19583564 | 0.61 | No |
| 77 | US 101 | McKay Creek Rd | Benham Ln | 1.9 | 10558 | Rural Cities | Other Principal Arterials | 34 | 34 | 1.06 | 35646448 | 0.95 | No |
| 78 | US 101 | Benham Ln | S Bank Chetco River Road | 1.3 | 17126 | Rural Cities | Other Principal Arterials | 45 | 46 | 1.06 | 39381237 | 1.17 | Yes |
|  | US 101 | S Bank Chetco River Road | Oak St | 0.6 | 20463 | Rural Cities | Other Principal Arterials | 39 | 40 | 1.06 | 20912675 | 1.91 | Yes |
| 79 | US 101 | Oak St | 5th St | 0.6 | 17302 | Rural Cities | Other Principal Arterials | 58 | 60 | 1.06 | 17998752 | 3.33 | Yes |
| 80 | US 101 | 5th St | Harris Park | 1.2 | 10836 | Rural Cities | Other Principal Arterials | 46 | 48 | 1.06 | 24324111 | 1.97 | Yes |
| 81 | US 101 | Harris Park | Longacre Loop | 1.6 | 7267 | Rural Cities | Other Principal Arterials | 14 | 14 | 1.06 | 20556526 | 0.68 | No |
| 82 | US 101 | Longacre Loop | UGB | 1.9 | 5501 | Rural Cities | Other Principal Arterials | 11 | 11 | 1.06 | 19375897 | 0.57 | No |
| 83 | US 101 | UGB | Cape Ferrelo Rd | 1.1 | 3808 | Rural Area | Other Principal Arterials | 5 | 7 | 1.22 | 7644560 | 0.92 | No |
|  | US 101 | Cape Ferrelo Rd | Martin Ranch Rd | 1.4 | 3808 | Rural Area | Other Principal Arterials | 12 | 12 | 1.22 | 9520952 | 1.26 | Yes |
| 84 | US 101 | Martin Ranch Rd | Bridgeview Ln | 2.6 | 3808 | Rural Area | Other Principal Arterials | 15 | 15 | 1.22 | 18346944 | 0.82 | No |
| 85 | US 101 | Bridgeview Ln | Hooskanaden Creek | 3.1 | 3808 | Rural Area | Other Principal Arterials | 23 | 24 | 1.22 | 21265776 | 1.13 | No |
| 86 | US 101 | Hooskanaden Creek | Byrdies Rd | 2.4 | 3621 | Rural Area | Other Principal Arterials | 2 | 18 | 1.22 | 15992147 | 1.13 | No |
| 87 | US 101 | Byrdies Rd | Carpenterville Hwy | 1.9 | 2642 | Rural Area | Other Principal Arterials | 1 (with Carpenterville) | 4 | 1.22 | 9161135 | 0.44 | No |
| 88 | US 101 | Carpenterville Hwy | N of Cape View Loop | 2.8 | 3743 | Rural Area | Other Principal Arterials |  | 17 | 1.22 | 19260776 | 0.88 | No |
| 89 | US 101 | N of Cape View Loop | Carpenterville Hwy N | 2.0 | 3968 | Rural Area | Other Principal Arterials | 0 | 16 | 1.22 | 14700448 | 1.09 | No |
| 90 | US 101 | Carpenterville Hwy N | UGB | 3.6 | 3968 | Rural Area | Other Principal Arterials | 3 | 18 | 1.22 | 25997344 | 0.69 | No |
| 91 | US 101 | UGB | Hunter Creek Rd | 0.2 | 4547 | Rural Cities | Other Principal Arterials | 1 | 2 | 1.06 | 1825621 | 1.10 | Yes |
| 92 | US 101 | Hunter Creek Rd | Hunter Creek Lp | 0.8 | 4841 | Rural Cities | Other Principal Arterials | 2 | 4 | 1.06 | 6979512 | 0.57 | No |
| 93 | US 101 | Hunter Creek Lp | Jerry's Flat Rd | 2.4 | 7842 | Rural Cities | Other Principal Arterials | 15 | 19 | 1.06 | 33917993 | 0.56 | No |
| 94 | US 101 | Jerry's Flat Rd | N Bank Rogue River Rd | 0.4 | 6510 | Rural Cities | Other Principal Arterials | 5 | 6 | 1.06 | 4871108 | 1.23 | Yes |
| 95 | US 101 | N Bank Rogue River Rd | UGB | 1.3 | 3888 | Rural Cities | Other Principal Arterials | 2 | 5 | 1.06 | 9224280 | 0.54 | No |
| 96 | US 101 | UGB | Old County Rd S | 2.1 | 3319 | Rural Area | Other Principal Arterials | 2 | 7 | 1.22 | 12841211 | 0.55 | No |
| 97 | US 101 | Old Coast Road S | Edson Creek Rd | 2.0 | 3319 | Rural Area | Other Principal Arterials | 5 | 10 | 1.22 | 1181491 | 0.85 | No |
| 98 | US 101 | Edson Creek Rd | Ophir Rd | 1.2 | 2930 | Rural Area | Other Principal Arterials | 3 | 4 | 1.22 | 6256283 | 0.64 | No |
| 99 | US 101 | Ophir Rd | Euchre Creek Rd | 4.0 | 2517 | Rural Area | Other Principal Arterials | 2 | 6 | 1.22 | 18374100 | 0.33 | No |
| 100 | US 101 | Euchre Creek Rd | Pacific Highland Dr | 5.0 | 2471 | Rural Area | Other Principal Arterials | 1 | 17 | 1.22 | 22547875 | 0.75 | No |
| 101 | US 101 | Pacific Highland Dr | China Mountain Rd | 4.9 | 2463 | Rural Area | Other Principal Arterials | 3 | 37 | 1.22 | 22025378 | 1.68 | Yes |
| 102 | US 101 | China Mountain Rd | UGB | 3.7 | 3122 | Rural Area | Other Principal Arterials | 0 | 10 | 1.22 | 20910376 | 0.48 | No |
| 103 | US 101 | UGB | Cemetary Loop Rd | 1.9 | 3122 | Rural Cities | Other Principal Arterials | 7 | 10 | 1.06 | 10939488 | 0.91 | No |
| 104 | US 101 | Cemetary Loop Rd | 9th St | 1.0 | 3546 | Rural Cities | Other Principal Arterials | 3 | 3 | 1.06 | 6147878 | 0.49 | No |
| 105 | US 101 | 9th St | UGB | 2.5 | 4831 | Rural Cities | Other Principal Arterials | 2 | 5 | 1.06 | 21951000 | 0.23 | No |
| 106 | US 101 | UGB | Elk River Rd | 0.6 | 3954 | Rural Area | Other Principal Arterials | 1 | 1 | 1.22 | 4329630 | 0.23 | No |
| 107 | US 101 | Elk River Rd | Cape Blanco Highway | 1.2 | 3816 | Rural Area | Other Principal Arterials | 1 | 2 | 1.22 | 8635608 | 0.23 | No |
| 108 | US 101 | Cape Blanco Highway | Sixes River Rd | 0.9 | 3646 | Rural Area | Other Principal Arterials | 2 | 4 | 1.22 | 6121634 | 0.65 | No |
| 109 | US 101 | Sixes River Rd | Airport Rd | 1.9 | 3665 | Rural Area | Other Principal Arterials | 4 | 13 | 1.22 | 12975933 | 1.00 | No |
| 110 | US 101 | Airport Rd | Floras Lake Loop Rd | 3.7 | 4006 | Rural Area | Other Principal Arterials | 5 | 25 | 1.22 | 26831187 | 0.93 | No |
| 111 | US 101 | Floras Lake Loop Rd | Langlois Mountain Rd | 2.2 | 4006 | Rural Area | Other Principal Arterials | 9 | 14 | 1.22 | 16157200 | 0.87 | No |
| 112 | US 101 | Langlois Mountain Rd | County Boundary (Coos) | 1.9 | 3766 | Rural Area | Other Principal Arterials | 2 | 7 | 1.22 | 12989876 | 0.54 | No |
| 113 | Cape Blanco Highway | US 101 | Western Terminus | 2.5 | 446 | Rural Area | Other Principal Arterials | 0 | 0 | 1.22 | 2034875 | 0.00 | No |
| 114 | OR 251 | US 101 | Western Terminus | 0.8 | 342 | Rural Area | Other Principal Arterials | 1 | 1 | 1.22 | 487406 | 2.05 | Yes |
| 115 | OR 255 | US 101 N | US 1015 | 3.6 | 87 | Rural Area | Other Principal Arterials | 1 | 3 | 1.22 | 565239 | 5.31 | Yes |
| 116 | OR 255 | Pistol River Loop | Mack Arch Rd/US 101 | 2.0 | 78 | Rural Area | Other Principal Arterials | 0 | 2 | 1.22 | 286124 | 6.99 | Yes |
| 117 | OR 255 | Mack Arch Rd/US 101 | Burnt Hill Dr | 3.7 | 78 | Rural Area | Other Principal Arterials | 0 | 0 | 1.22 | 528119 | 0.00 | No |
| 118 | OR 255 | Burnt Hill Dr | Bull Gulch Rd | 1.9 | 78 | Rural Area | Other Principal Arterials | 0 | 0 | 1.22 | 266195 | 0.00 | No |
| 119 | OR 255 | Bull Gulch Rd | Bosley Butte Rd | 1.8 | 67 | Rural Area | Other Principal Arterials | 0 | 1 | 1.22 | 213981 | 4.67 | Yes |
| 120 | OR 255 | Bosley Butte Rd | Whaleshead Rd | 3.2 | 67 | Rural Area | Other Principal Arterials | 0 | 0 | 1.22 | 393726 | 0.00 | No |
| 121 | OR 255 | Whaleshead Rd | Johns Pl | 2.2 | 67 | Rural Area | Other Principal Arterials | 1 | 3 | 1.22 | 272673 | 11.00 | Yes |
| 122 | OR 255 | Johns PI | Duley Creek Rd | 2.8 | 249 | Rural Area | Other Principal Arterials | 1 | 9 | 1.22 | 1254213 | 7.18 | Yes |
| 123 | OR 255 | Duley Creek Rd | UGB | 2.6 | 249 | Rural Area | Other Principal Arterials | 0 | 6 | 1.22 | 1199682 | 5.00 | Yes |
| 124 | OR 255 | UGB | US 101 | 0.5 | 1956 | Rural Cities | Other Principal Arterials | 5 | 8 | 1.06 | 1927145 | 4.15 | Yes |

## ATTACHMENT E - BICYCLE LEVEL OF TRAFFIC STRESS WORKSHEETS



## ATTACHMENT F - PEDESTRIAN QUALATATIVE MULTIMODAL ASSESSMENT WORKSHEETS

| Street | From | то | Outside Travel Lane Width |  | Bike Lane/Shoulder Width |  | Presence of Buffer |  | Sidewalk/Path Presence |  | Lighting |  | Travel Lanes and Vehicle Speed |  |  | Average Score | PedQMA |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Width (Feet) | Score | Width (Feet) | Score | Width (Feet) | Score | Type | Score | Type | Score |  |  |  |  |  |
| 1 Winchuck River Rd | US 101 | Mp 7.0 | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Sparse Lighting | 2 | 2 lanes | $40+$ | 1 | 1.5 | Poor |
| 2 Winchuck River Rd | MP 7.0 | Wheeler Creek Rd | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 3 oceanview Dr | Winchuck River Rd | Seagull L | 11 | 2 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | Sparse Lighting | 2 | 2 lanes | $40+$ | 1 | 1.8 | Poor |
| 4 Oceanview Dr | Seagull L | Benham Ln | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Sparse Lighting | 2 | 2 lanes | 35 | 2 | 1.5 | Poor |
| 5 w Benham Ln | Oceanview Dr | Oisen Ln | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Sidewalk | 3 | Sparse Lighting | 2 | 2 lanes | 30 | 3 | 2.5 | Fair |
| 6 w Benham Ln | Oisen Ln | US 101 | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Sidewalk | 3 | Sparse Lighting | 2 | 2 lanes | 30 | 3 | 2.5 | Fair |
| 75 Bank Chetco River Rd | HwY 009 | 5 Bank Chetco Underpass | 12 | 3 | >6 | 4 | 0 | 1 | Sidewalk | 3 | Sparse Lighting | 2 | 2 lanes | 35 | 2 | 2.5 | Fair |
| 8 s Bank Chetco River Rd | 5 Bank chetco Underpass | Payne Road | 12 | 3 | >6 | 4 | 0 | 1 | Sidewalk | 3 | No lighting | 1 | 2 lanes | ${ }^{40+}$ | 1 | 2.2 | Fair |
| 9 S Bank Chetco River Rd | Payne Road | MP 4.0 | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 10 S Bank Cheto River Rd | MP 4.0 | Mt Emily Trail | 10 | 1 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 1.0 | Poor |
| ${ }_{11}$ Lower Harbor Rd | Benham Ln | US 101 | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | Sparse Lighting | 2 | 2 lanes | 30 | 3 | 2.3 | Fair |
| 12 Shopping Center Ave | W Hoffeldt Ln | Lower Harbor Rd | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Sidewalk | 3 | Sparse Lighting | 2 | 2 lanes | 35 | 2 | 2.3 | Fair |
| 13 N Bank Chetco River Rd | Old County Road | MP 5.0 | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | ${ }^{40+}$ | 1 | 1.3 | Poor |
| 14 N Bank Chetco River Rd | MP 5.0 | Gardner Ridge Rd | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| 15 n Bank Chetco River Rd | Garder Ridge Rd | MP 17.5 | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | ${ }^{40+}$ | 1 | 1.2 | Poor |
| 16 Gardiner Ridge Rd | N Bank Chetco River Rd | MP 17.0 | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes |  | NA | 1.2 | Poor |
| 17 Cape Ferrelo Rd | US 101 | Sundown Rd | 12 | ${ }^{3}$ | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 |  | NA | 2 lanes | 35 | 2 | 1.6 | Poor |
| 18 Cape Ferrelo Rd | Sundown Rd | HWY 255 | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 |  | NA | 2 lanes | 35 | 2 | 1.6 | Poor |
| 19 Pistol River Loop | HwY 255 | Hwy 255 (Cape View Loop) | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 20 NBank Pistol River Rd | Pistol River Loop | MP 8 (Forest Boundary) | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 |  | NA | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| 21 Hunter Creek Rd | HWY 009 | Meyers Rd | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 |  | NA | 2 lanes | $40+$ | 1 | 1.4 | Poor |
| 22 Hunter Creek Rd | MP 2.5 | Little South Fork Rd | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 23 Nf-3680 | Hunter Creek Rd | Agness Rd | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 24 Jerry Flat Rd | US 101 | Eagleview DR (MP 80.0) | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | Nolighting | 1 | 2 lanes | 30 | 3 | 2.2 | Fair |
| 25 Jerry Flat Rd | Eagleview DR (MP 80.0) | UGB (MP 75.0) | 12 | 3 | 2 to 4 | 2 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | 35 | 2 | 1.7 | Poor |
| 26 Jerry Flat Rd | UGB (MP 75.0) | Lobster Creek Rd | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | 35 | 2 | 1.5 | Poor |
| ${ }^{27}$ Agness Rd | Lobster Creek Rd | Galice Creek Rd | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| 28 Agness Rd | Galice Creek Rd | County Boundary (Coos) | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| 29 Oak Flat Rd | Agness Rd | campground road (MP 3) | 11 | 2 | <2 | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| ${ }^{30}$ Galice Creek Rd | Agness Rd | County Boundary (Josephine) | 10 | 1 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 |  | NA | 2 lanes | $40+$ | 1 | 1.0 | Poor |
| ${ }^{31} \mathrm{~N}$ Bank Rogue River Rd | US 101 | Krysten Ln | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 32 N Bank Rogue River Rd | krysten Ln | Lobster Creek Rd | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 33 Cedar Valley Rd | N Bank Rogue River Rd | Ophir Rd | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 |  | NA | 2 lanes | $40+$ | 1 | 1.4 | Poor |
| 34 Edson Creek Rd | N Bank Rogue River Rd | us 101 | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 35 Lobster Creek Rd | N Bank Rogue River Rd | Agness Rd | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| 36 Nesika Beach Rd | US 101 | US 101 | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| 37 Ophir Rd | US 101 | Euchre Creek Rd | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 |  | NA | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| 38 Euchre Creek Rd | Ophir Rd | MP 3 (Forest Road) | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 39 Paradise Point Rd | US 101 | Paradise Point State Park | 11 | , | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes |  | NA | 1.2 | Poor |
| 40 HW 251 (Port Offord HWY) | 9th Street | Coast Guard Hill Rd | 10 | 1 | $<2$ | 1 | 2-4 | 3 | Sidewalk | 3 | Nolighting | 1 | 2 lanes | 25 | 4 | 2.2 | Fair |
| 41 Elk River Rd | US 101 | Wagner Ln | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 42 Elk River Rd | Wagner Ln | Haiku Ln (MP 5.5) | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 43 Elk River Rd/NF 5325 | Haiku Ln (MP 5.5) | County Boundary (Coos) | 12 | ${ }^{3}$ | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 44 Cape Blanco | Cape Blanco Hwy | MP 3 | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 |  | NA | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| 45 Sixes River Rd | US 101 | NF-4600 | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 46 Airport Rd | US 101 | The Airport | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes |  | NA | 1.4 | Poor |
| 47 Floras Lake Loop Rd | US 1015 | Us 101 N | 12 | 3 | <2 | 1 | 0 | 1 | No Sidewalk | 1 | Sparse Lighting | 2 | 2 lanes |  | NA | 1.6 | Poor |
| 48 Floras Lake Rd | Floras Lake Loop Rd | Lakes End Dr | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 |  | NA | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| 49 Floras Creek Rd | US 101 | Allen Canyon Loop | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 50 Floras Creek Rd | Allen Canyon Loop | S Fork Flores Creek Rd | 12 | 3 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 51 Langlois Mountain Rd | US 101 | Bethel Creek Rd | 11 | 2 | <2 | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| 52 US 101 | CA border | Itzen Dr | 12 | 3 | >6 | 4 | 0 | 1 | Paved Shoulder | 2 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 2.0 | Fair |
| 53 US 101 | Itzen Dr | Freeman Mt Ln | ${ }^{12+}$ | 4 | >6 | 4 | 0 | 1 | Paved Shoulder | 2 | No lighting | 1 | 2 lanes | $40+$ | 1 | 2.2 | Fair |
| 54 US 101 | Freeman Mt $\llcorner$ n | Raymond Ln | 12 | 3 | >6 | 4 | 0 | 1 | Paved Shoulder | , | Sparse Lighting | 2 | >2 lanes | $40+$ |  | 2.2 | Fair |
| 55 US 101 | Raymond Ln | Benham Ln | 12 | 3 | >6 | 4 | 0 | 1 | Paved Shoulder | 2 | Sparse Lighting | 2 | >2 lanes | $40+$ | 1 | 2.2 | Fair |
| 56 US 101 | Benham Ln | N Bank Chetoo River Rd | 12 | 3 | >6 | 4 | 0 | 1 | Sidewalk |  | Sparse Lighting | 2 | >2 lanes | $40+$ | 1 | 2.3 | Fair |
| 57 US 101 | N Bank Chetco River Rd | Oakst | 12 | 3 | $>6$ | 4 | 0 | 1 | Sidewalk | 3 | Typical Lighting | 3 | >2 lanes | 35 | 2 | 2.7 | Fair |
| 58 US 101 | Oak St | Wharf St (Center St) | 11 | 2 | $>6$ | 4 | $2-4$ | 3 | Sidewalk | 3 | Typical Lighting | 3 | >2 lanes | 25 | 4 | 3.2 | Good |
| 59 us 101 | Wharf St | Pacific Ave | ${ }^{12+}$ | 4 | 4 to 6 | 3 | $2-4$ | 3 | Sidewalk | 3 | Typical Lighting | 3 | >2 lanes | 35 | 2 | 3.0 | Good |
| 60 U 101 | Pacific Ave | 5 5th 5 t | 12 | 3 | 4 to 6 | 3 | $2-4$ | 3 | Sidewalk | 3 | Typical Lighting | 3 | $>2$ lanes | 35 | 2 | 2.8 | ${ }^{\text {Fair }}$ |
| 61 US 101 | 5 th St | Easy St | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Sidewalk | 3 | Typical Lighting | ${ }^{3}$ | $>2$ lanes | 35 | 2 | 2.5 | Fair |
| 62 US 101 | Easty St | Parkview Dr | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Sidewalk | 3 | Sparse Lighting | 2 | 2 lanes | 35 | ${ }_{2}$ | 2.3 | Fair |
| 63 US 101 | Parkview Dr | north of Harris Park | ${ }^{12+}$ | 4 | >6 | 4 | 0 | 1 | Paved Shoulder | 2 | No lighting | 1 | 2 lanes | $40+$ | 1 | 2.2 | Fair |
| 64 US 101 | north of Harris Park | south of OR 255 | 12 | 3 | >6 | 4 | 0 | 1 | Paved Shoulder | 2 | No lighting | 1 | 2 lanes | $40+$ | 1 | 2.0 | Fair |
| 65 US 101 | south of OR 255 | Longarce Lp | ${ }^{12+}$ | 4 | $>6$ | 4 | 0 | 1 | Paved Shoulder | 2 | No lighting | 1 | 2 lanes | $40+$ | 1 | 2.2 | Fair |
| 66 US 101 | Longarce Lp | south of House Rock Rd | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | No lighting | 1 | >2 lanes | $40+$ | 1 | 1.8 | Poor |
| 67 US 101 | south of House Rock Rd | Cape Ferrelo Rd | 11 | 2 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | Nolighting | 1 | >2 lanes | ${ }^{40+}$ | 1 | 1.7 | Poor |
| 68 US 101 | Cape Ferrelo Rd | McDonald Rd | 12 |  | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | No lighting | 1 | >2 lanes | $40+$ | 1 | 1.8 | Poor |
| 69 US 101 | McDonald Rd | 1.5Miles N of Widerness Road (turns to t. | 12 | 3 | $>6$ | 4 | 0 | 1 | Paved Shoulder | 2 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 2.0 | Fair |
| 70 US 101 | 1.5miles N of Wilderness Road (turns to | Mack Conn. (OR 225) | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | Nolighting | 1 | >2 lanes | $40+$ | 1 | 1.8 | Poor |
| 71 US 101 | OR 255 | Birdies Ln | 11 | 2 | >6 | 4 | 0 | 1 | Paved Shoulder | 2 | Nolighting | 1 | 2 lanes | $40+$ |  | 1.8 | Poor |
| 72 US 101 | Birdies Ln | Meyers Creek Conn. (OR 255) | 12 | 3 | $>6$ | 4 | 0 | 1 | Paved Shoulder | 2 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 2.0 | Fair |
| 73 US 101 | Meyers Creek Conn. (OR 255) | north of Meyers Creek Conn. (OR 255) | ${ }^{12+}$ |  | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | Nolighting | 1 | >2 lanes | $40+$ | 1 | 2.0 | Fair |
| 74 US 101 | north of Meyers Creek Conn. (OR 255) | Herman Ln | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | No lighting | 1 | >2 lanes | ${ }^{40+}$ | 1 | 1.8 | Poor |
| 75 us 101 | Herman Ln | Bellview Ln | ${ }^{12+}$ | 4 | 4 to 6 |  | 0 | 1 | Paved Shoulder |  | No lighting | 1 | >2 lanes | $40+$ | 1 | 2.0 | Fair |
| 76 US 101 | Bellview Ln | Kissing Rock Rd | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | No lighting | 1 | >2 lanes | $40+$ | 1 | 1.8 | Poor |
| 77 US 101 | Kissing Rock Rd | Hunter Creek Conn. | ${ }^{12+}$ | 4 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | Nolighting | 1 | $>2$ lanes | ${ }^{40+}$ | 1 | 2.0 | Fair |
| 78 US 101 | Hunter Creek rd | Kerber Dr | 12 | 3 | $>6$ | 4 | 0 | 1 | Paved Shoulder | 2 | No lighting |  | 2 lanes | ${ }^{40+}$ | 1 | 2.0 | Fair |
| 79 US 101 | Kerber Dr | Weber Way | 12 | 3 | $<2$ | 1 | 0 | 1 | Sidewalk | 3 | Typical Lighting | 3 | $>2$ lanes | 30 | 3 | 2.3 | Fair |
| 80 US 101 | Weber Way | Moore st | ${ }^{12+}$ | 4 | $<2$ | 1 | 0 | 1 | Sidewalk | 3 | Typical Lighting | 3 | >2 lanes | 30 | 3 | 2.5 | Fair |
| 81 US 101 | Moore St | Jerry's Flat Rd | 12 | 3 | $<2$ |  | 0 | 1 | Sidewalk | 3 | Typical Lighting | 3 | >2 lanes | 30 | 3 | 2.3 | ${ }_{\text {Frair }}$ |
| 82 US 101 | Jerry's Flat Road | Wedderburn Loop Rd | ${ }^{12+}$ | 4 | 4 to 6 | 3 | 0 | 1 | Sidewalk | 3 | Nolighting | 1 | $>2$ lanes | ${ }^{40+}$ | 1 | 2.2 | Fair |
| 83 US 101 | Wedderburn Loop Rd | Ophir Rd | 12 | 3 | ${ }^{>} 6$ | 3 | 0 | 1 | Paved Shoulder | ${ }_{2}$ | Nolighting | 1 | 2 2lanes | ${ }_{40+}^{40+}$ | 1 | 2.0 | ${ }_{\text {Fair }}$ |
| 84 US 101 | Ophir Rd | 1 mile south of Humbug State Park Entran | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 1.8 | Poor |
| 85 Us 101 | 1mile south of Humbug State Park Entra | I north of Humbug Mt Front Rd | 12 | 3 | 2 to 4 | 2 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | ${ }_{40+}^{40+}$ | 1 | 1.5 | Poor |
| 86 US 101 | north of Humbug Mt Front Rd | 0.5m NW of Humbug State park entrance ¢ Rocky Point Bridge | 11 | 2 | 2 to 4 | 2 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 1.3 | Poor |
| 87 US 101 88 us 101 | 0.5m NW of Humbug State park entranci | ¢ Rocky Point Bridge | 12 | 3 | $\stackrel{4}{ }+6$ | 4 | 0 | 1 | Paved Shoulder Paved Shoulder | 2 | Noliehting | 1 | 2 2lanes | ${ }_{40+}^{40+}$ | 1 | 1.8 | $\underset{\text { Poor }}{\text { Fair }}$ |
| 89 US 101 90 US 101 | Fir Rd N Cemetary loop Rd | N Cemetary loop Rd Port Oford loop Rd | 12 12 | 3 |  | 3 4 | 2-4 | 1 3 | Paved Shoulder | 3 | Nol $\begin{aligned} & \text { Nolighting } \\ & \text { Typical Lighting }\end{aligned}$ | 3 | >2 lanes | $40+$ 30 | 1 | 1.8 3.2 | Poor Good |
| ${ }^{90}$ US 101 | N Cemetary loop Rd Port Oford loop Rd | Port Offord loop Rd Sixes River Rd | 12 12 | 3 3 | >6 | 4 | $2-4$ | 3 1 | Sidewalk Paved Shoulder | 3 2 2 | Typical Lighting No lighting | 3 1 1 | $>2$ lanes $>2$ lanes | 30 $40+$ 4 | 3 | 3.2 2.0 | $\underset{\substack{\text { Good } \\ \text { Fair }}}{\text { der }}$ |
| 92 US 101 | Sixes River Rd | S Floras Lake Loop Rd | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | 2 | No lighting | 1 | 2 lanes | $40+$ | 1 | 1.8 | Poor |
| 93 US 101 | S Floras Lake Loop Rd | Langlois Mountain Rd | 12 | 3 | >6 |  | 0 |  | Paved Shoulder | 2 | No lighting | 1 | 2 lanes | $40+$ | 1 | 2.0 | Fair |
| 94 US 101 | Langlois Mountain Rd | County Boundary (New Lake Ln) | 12 | 3 | 4 to 6 | 3 | 0 | 1 | Paved Shoulder | ${ }_{2}$ | Nolighting | 1 | 2 lanes | ${ }^{40+}$ | 4 | 1.8 | Poor |
| 95 Pedrioli Dr | Western Terminus | Ocean View Dr | ${ }_{11}^{11}$ | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | 25 | 4 | 1.7 | Poor |
| 96 Pedrioli Dr | Ocean View Dr | US 101 | ${ }_{11}^{11}$ | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Noliehting | 1 | 2 2lanes | 35 | 2 | 1.3 | Poor Poor |
|  | South of Titus Ln | US 101 | ${ }_{11}^{11}$ | ${ }_{2}$ | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | 25 | 4 | 1.7 | Poor |
| 98 Old Countr Md | Pacific Terrace Loop | UGB | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | 35 35 | 2 | 1.3 | Poor Poor |
| 99 Old County Rd | UGB | Eastern Terminus | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | 35 | 4 | 1.3 | Poor |
| 100 Parkview Dr | Vistra Ridge Dr ${ }^{\text {cas }}$ | Eastern Terminus | ${ }_{11}^{11}$ | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Noliehting | 1 | 2 lanes | 25 $40+$ | 4 | 1.7 | Poor |
| 101 Rainbow Rock Rd | Carpenterville Hwy | Aqua Vista Ln | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 2lanes | ${ }_{40+}^{40+}$ | 1 | 1.2 | Poor |
| 102 Rainbow Rock Rd | Aqua Vista Ln | Carpentervill H Hy | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Noliehting Nolighting | 1 | 2 2lanes | ${ }^{40+}$ | 1 | 1.2 |  |
| 103 Grizly Mountain Rd 104 Wedderburn Loop | UGB US 101 | Eastern Terminus Dovil Point Rd | ${ }_{11}^{11}$ | 2 | <2 | 1 | 0 | 1 | No Sidewalk | 1 | Nol lighting | 1 | 2 lanes | 35 | 2 | 1.3 | Poor |
| 104 Wedderburn Loop 105 Wedderburn Loop | US 101 Dovle Point Rd | Doyle Point Rd Old Coast Hwy | ${ }_{11}^{11}$ | 2 | 4 to 6 | 3 | 0 | 1 | No Sidewalk No Sidewalk | 1 | Sparse Lighting | 2 | ${ }_{2} 2$ lanes | 30 30 | 3 | 2.0 | Fair Poor |
| 106 Old Coast Hwy | Doyle Point Rd Wedderburn Loop | Old Coast Hwy Us 101 | 11 11 | 2 | ${ }_{2}^{2 \text { to }} \times 2$ | 2 | ${ }_{0}$ | 1 | No Sidewalk No Sidewalk | 1 | Sparss Lighting $\begin{gathered}\text { No lighting }\end{gathered}$ | 1 | 2 2lanes | ${ }_{40+}$ | 3 1 1 | 1.8 1.2 | Poor Poor |
| 107 China Mountain Rd | UGB | Us 101 | 11 | 2 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | $40+$ | 1 | 1.2 | Poor |
| 108 Cemetery Loop Rd | US 101 | US 101 | 11 | 2 | $<2$ | 1 | 0 | , | No Sidewalk | 1 | No lighting | 1 | 2 lanes | 35 | 2 | 1.3 | Poor |
| 109 Vista Dr | Gold Run Rd | Old Mill Rd | 11 | , | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 lanes | 35 | 2 | 1.3 | Poor |
| 110 Grass Knob Rd | US 101 | Eastern Terminus | 11 |  | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | No lighting | 1 | 2 lanes | ${ }^{40+}$ | 1 | 1.2 | Poor |
| 111 OR 255 |  |  | 10 | 1 | $<2$ | 1 | 0 | 1 | No Sidewalk | 1 | Nolighting | 1 | 2 2lanes | $40+$ 25 | 4 | 1.0 | Poor <br> Poor |
| OR 251 OR 250 |  |  | 10 11 | 1 | <2 | 1 | 0 | 1 |  | NA | Nol lighting No lighting | 1 | 2 lanes 2 lanes | 25 $40+$ | 4 | 1.6 1.2 | Poor Poor |

## ATTACHMENT G - TRANSIT QUALITATIVE MULTIMODAL ASSESSMENT WORKSHEETS



## Schedule Speed \& Travel Time

| Route | Time | Excellent | Good | Fair | Poor |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | <20\% slower than driving | 20-40\% slower than driving | 40-60\% slower than driving | >60\% slower than driving |
| CPT | AM Peak |  | 3 |  |  |
| CPT | Mid-Day |  | 3 |  |  |
| CPT | PM peak |  | 3 |  |  |
| SouthWEST Point | Mid-Day | 4 |  |  |  |
|  |  |  |  |  |  |
|  |  |  |  |  |  |
| Transit Stop Amenities |  |  |  |  |  |
| Route | Stop | Excellent | Good | Fair | Poor |
|  |  | Shelter | Bench | Sign with waiting area | Sign with no waiting area and/or no sign |
| CPT | Harbor (Chevron) |  |  |  | 1 |
| CPT | Harbor (Umpqua Bank) |  |  |  | 1 |
| CPT | Brookings (5th St/Bankus Park) | 4 |  |  |  |
| CPT | Gold Beach (Ray's) | 4 |  |  |  |
| CPT | Port Orford (Ray's) | 4 |  |  |  |
| CPT | Langlois Public Library (Flag Stop) |  |  |  | 1 |
| CPT | Langlois Store (Flag Stop) |  |  |  | 1 |
| SouthWEST Point | Brookings (5th St/Bankus Park) | 4 |  |  |  |


| Connecting to Pedestrian : | Stop | Column3 | Column4 | Column5 | Column6 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Route | Stop | Excellent | Good | Fair | Poor |
|  |  | Bike lanes, sidewalks, or shoulders and crossing | Bike lanes, sidewalks, or shoulders, no crossing | Narrow bike lanes, sidewalks, or shoulders, no crossing | No sidewalks/bike lanes/ shoulders or crossing |
| CPT | Harbor (Chevron) |  |  | 2 |  |
| CPT | Harbor (Umpqua Bank) |  |  | 2 |  |
| CPT | Brookings (5th St/Bankus Park) | 4 |  |  |  |
| CPT | Gold Beach (Ray's) |  |  | 2 |  |
| CPT | Port Orford (Ray's) |  |  | 2 |  |
| CPT | Langlois Public Library (Flag Stop) |  |  |  | 1 |
| CPT | Langlois Store (Flag Stop) |  |  |  | 1 |


| Column1 | Column2 | Column3 | Column4 | Column5 | Column6 | Column7 | Column 8 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Freqency and On-Time |  |  | Connecting to Pedestrian \& |  |  |
| Route | Time | Reliability | Schedule Speed \& Travel Time | Transit Stop Amenities | Bicycle Network | Average | Rating |
| CPT | AM Peak | 1.000 | 3.000 | 2.286 | 2.00 | 2.07 | Fair |
| CPT | Mid-Day | 1.000 | 3.000 | 2.286 | 2.00 | 2.07 | Fair |
| CPT | PM peak | 1.000 | 3.000 | 2.286 | 2.00 | 2.07 | Fair |
| SouthWEST Point | Mid-Day | 1.000 | 4.000 | 4.000 | 4.00 | 3.25 | Good |


[^0]:    5 | Curry County Transportation System Plan | Kittelson \& Associates, Inc.

[^1]:    7 | Curry County Transportation System Plan | Kittelson \& Associates, Inc.

[^2]:    12 | Curry County Transportation System Plan | Kittelson \& Associates, Inc.

[^3]:    15 | Curry County Transportation System Plan
    Kittelson \& Associates, Inc.

[^4]:    20 | Curry County Transportation System Plan | Kittelson \& Associates, Inc.

[^5]:    24 | Curry County Transportation System Plan | Kittelson \& Associates, Inc.

[^6]:    Comments:

